



DeepEX
Shoring Design Software

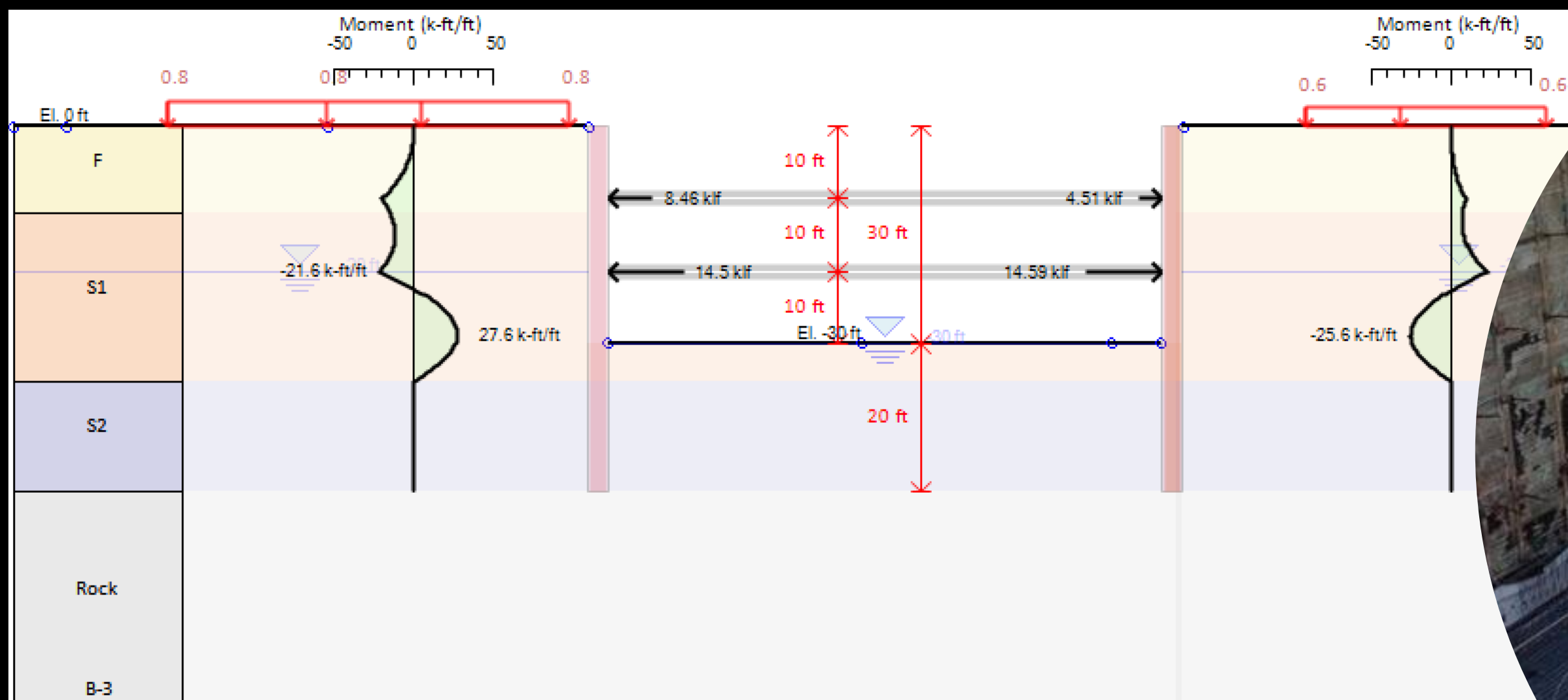
Design of Deep Excavations DeepEX Software Application



DEEP EXCAVATION
GEOTECHNICAL SOFTWARE & SOLUTIONS

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Seminar Notes & Demo: www.deepexcavation.com/en/workshops-seminars/seminar_auth





DeepEX
Shoring Design Software

Our Company

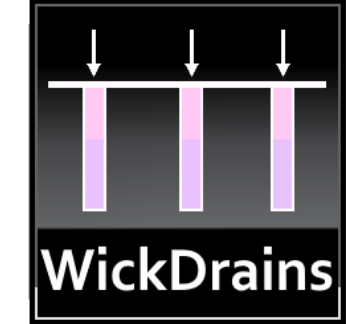
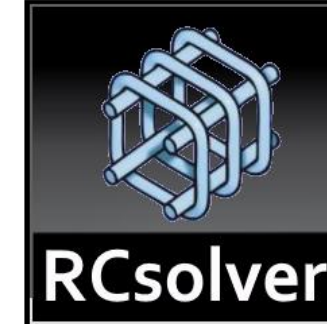
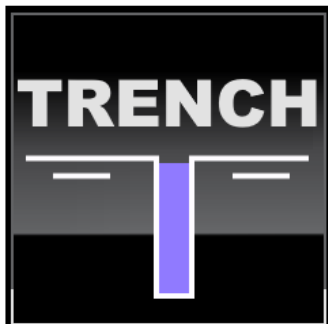


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www.deepex.com

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- ✓ Software solutions for excavation and foundation professionals
- ✓ Consulting Services - Design of deep excavations and pile foundations
- ✓ Virtual Reality applications for geotechnical engineers and contractors





Introduction to Deep Excavations - DeepEX Features

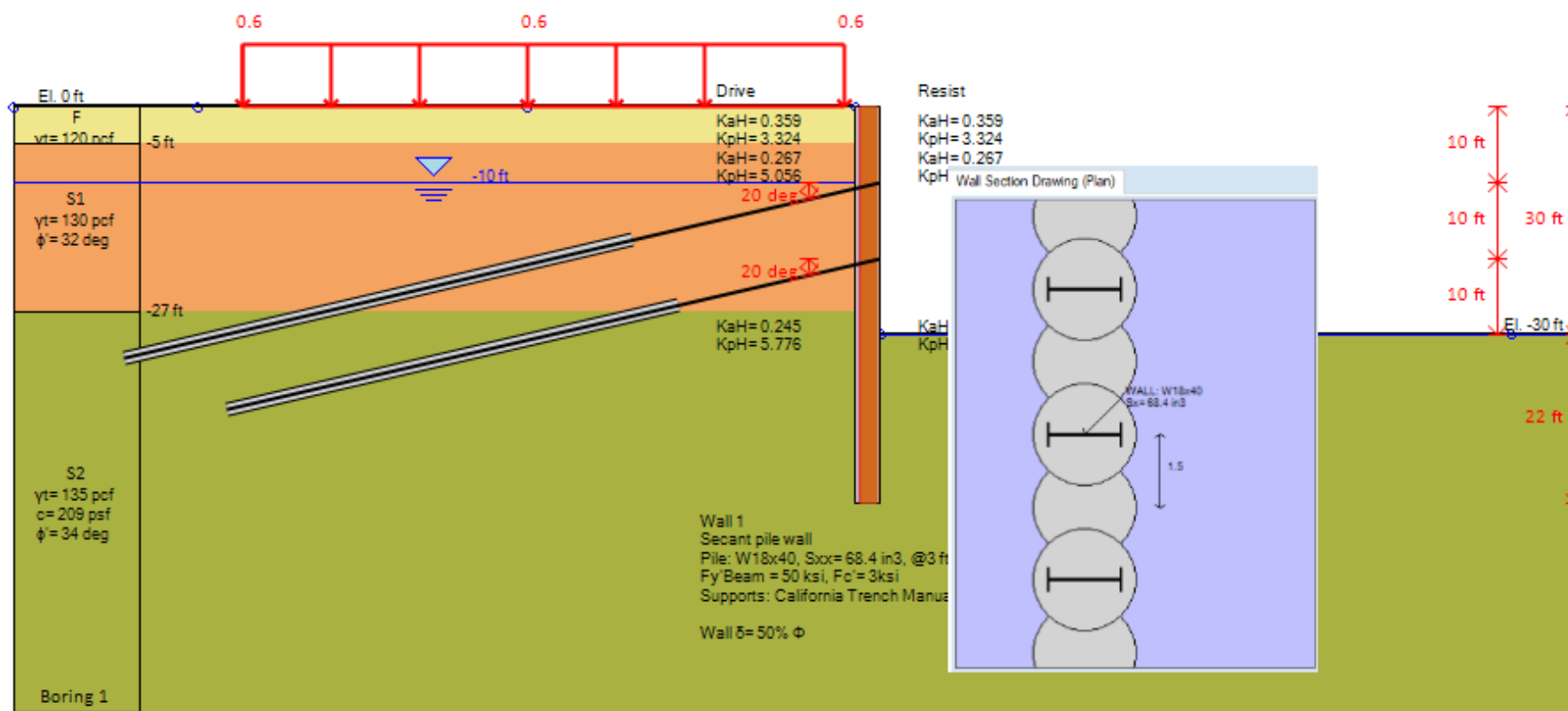
DEEP EX DeepEX
Superior Software Solution
for Excavation Professionals

Access deepexcavation.com
DeepEX Features & Capabilities



Deep Excavation: Typically deeper than 3.5m, that requires structural support.

- A deep excavation system has to retain earth, water, and neighboring structures
- Unknown factors and risks
- Soil properties estimation
- Protect adjacent properties
- Design issues and Code issues
- Economy
- Constructability





Full Structural and Geotechnical Design of any Deep Excavation Model



Wall Types in DeepEX

- ✓ Soldier Pile and Lagging Walls
- ✓ Sheet Pile Walls
- ✓ Secant / Tangent Pile Walls
- ✓ Concrete Diaphragm Walls (Slurry Walls)
- ✓ Soldier Pile and Tremied Concrete Walls
- ✓ Combined Sheet Pile Walls (King Piles)
- ✓ Box Sheet Pile Walls
- ✓ Custom Walls

Support Systems in DeepEX

- ✓ Anchored Walls (Tiebacks and Helical Anchors)
- ✓ Braced Excavations (Steel Struts and Rakers)
- ✓ Top/Down Excavations with Concrete Slabs
- ✓ Dead-man Walls
- ✓ Bin-Type Walls
- ✓ Cofferdams
- ✓ Circular Shafts
- ✓ Cantilever Walls

ANALYSIS METHODS: LIMIT EQUILIBRIUM ANALYSIS



Soil Pressures: Active/Passive, At-rest, Apparent Pressures (FHWA, Peck, Adaptive, Custom Trapezoidal +more)

Beam Analysis: Blum's, FHWA Simple Span, CALTRANS +more

NON-LINEAR ANALYSIS (SOIL SPRINGS)



Moments and Reactions from Spring Analysis

Cumulative Results from Stages

Realistic Displacements

FINITE ELEMENT ANALYSIS

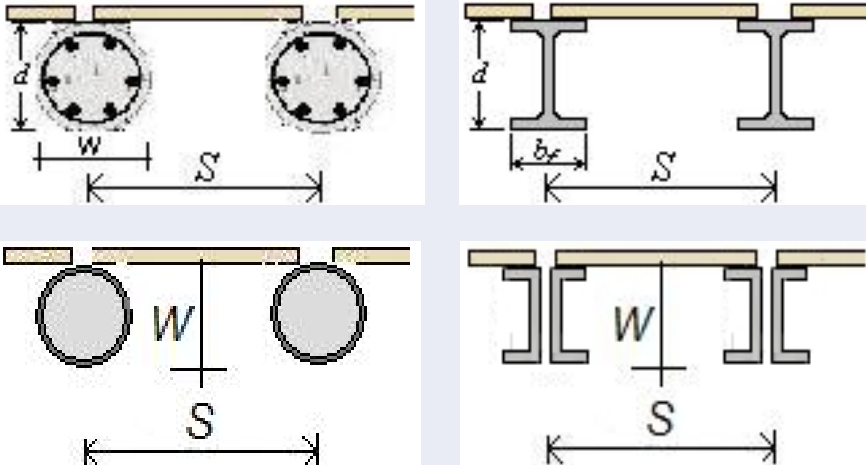
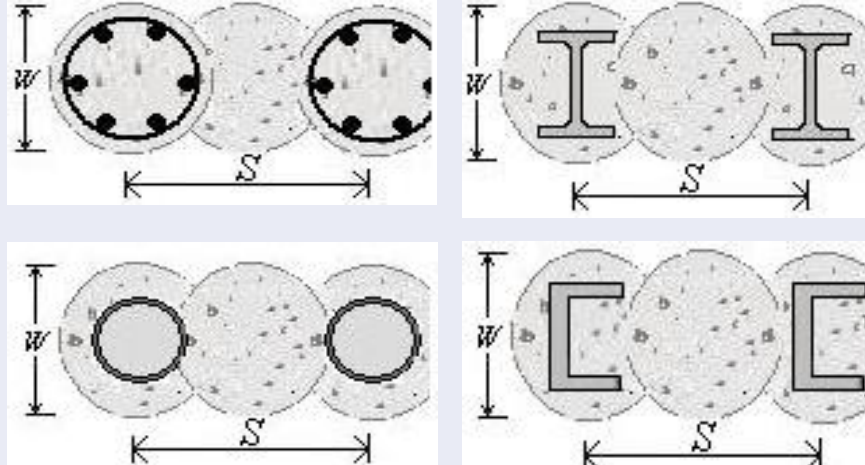
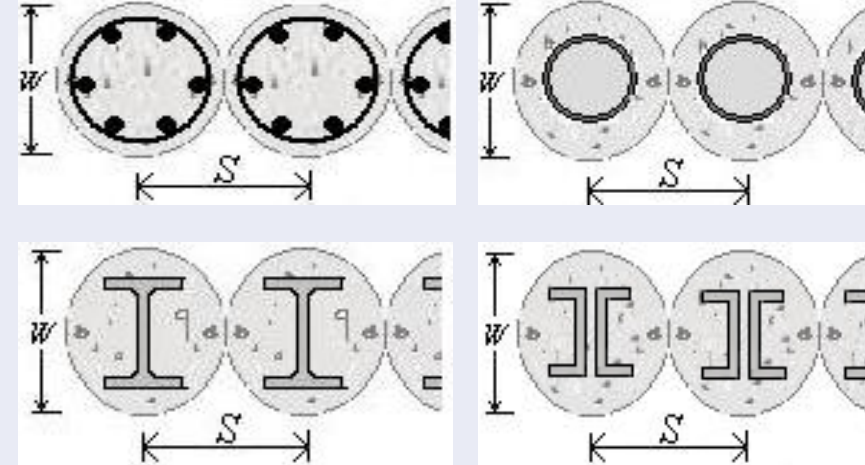
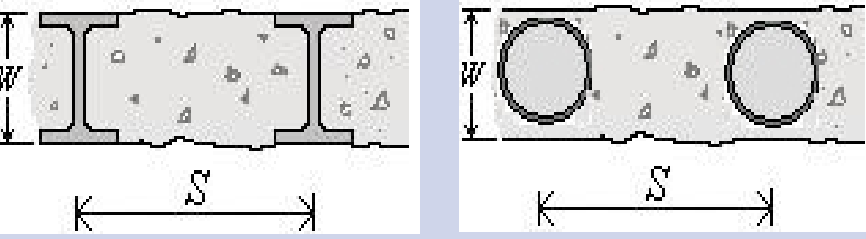
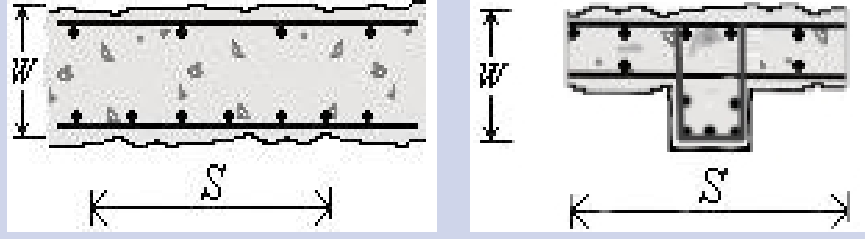
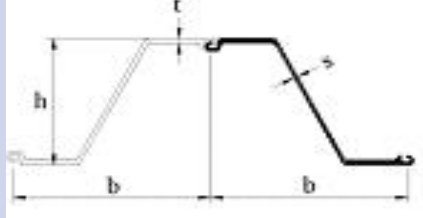
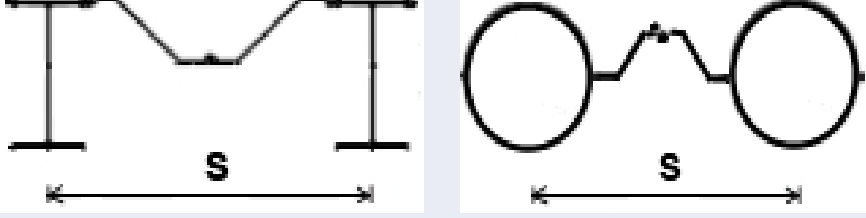
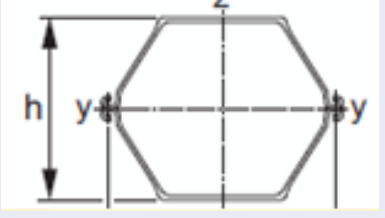
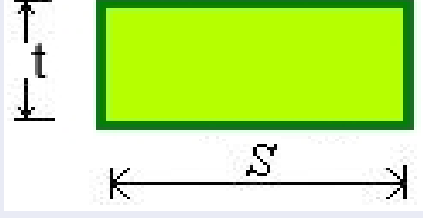


Moments and Reactions from Finite Elements

Full Soil-Structure Interaction

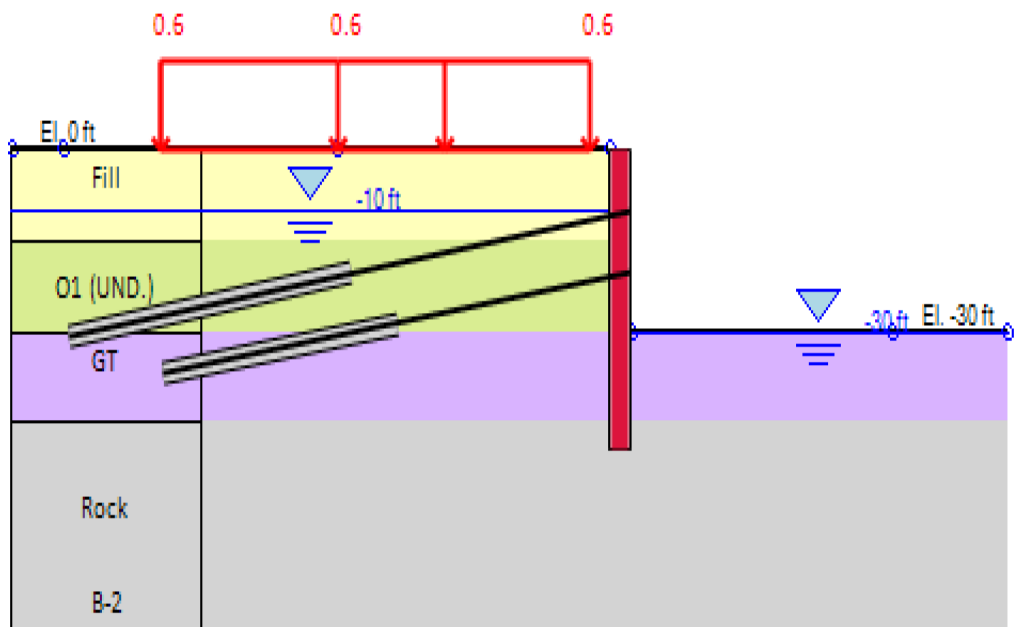
Calculate Surface Settlements



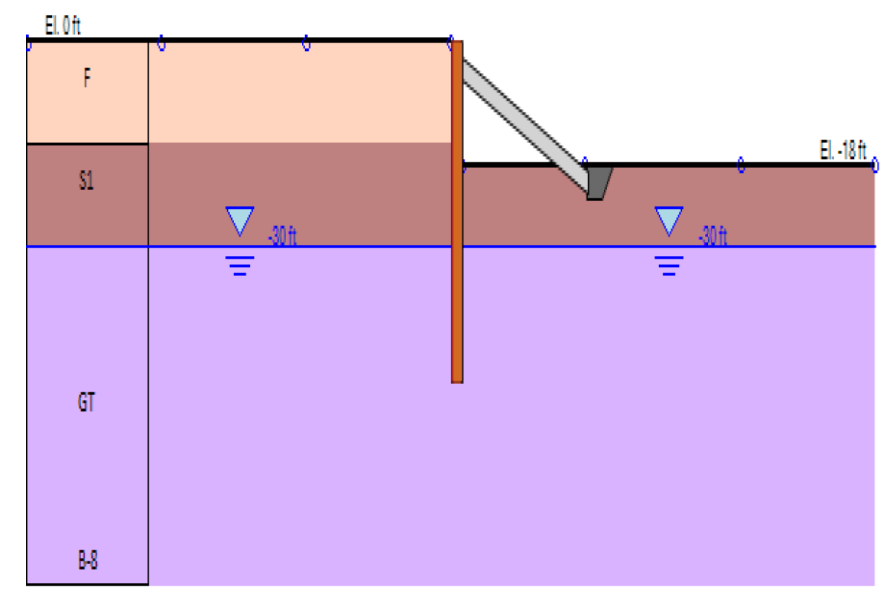
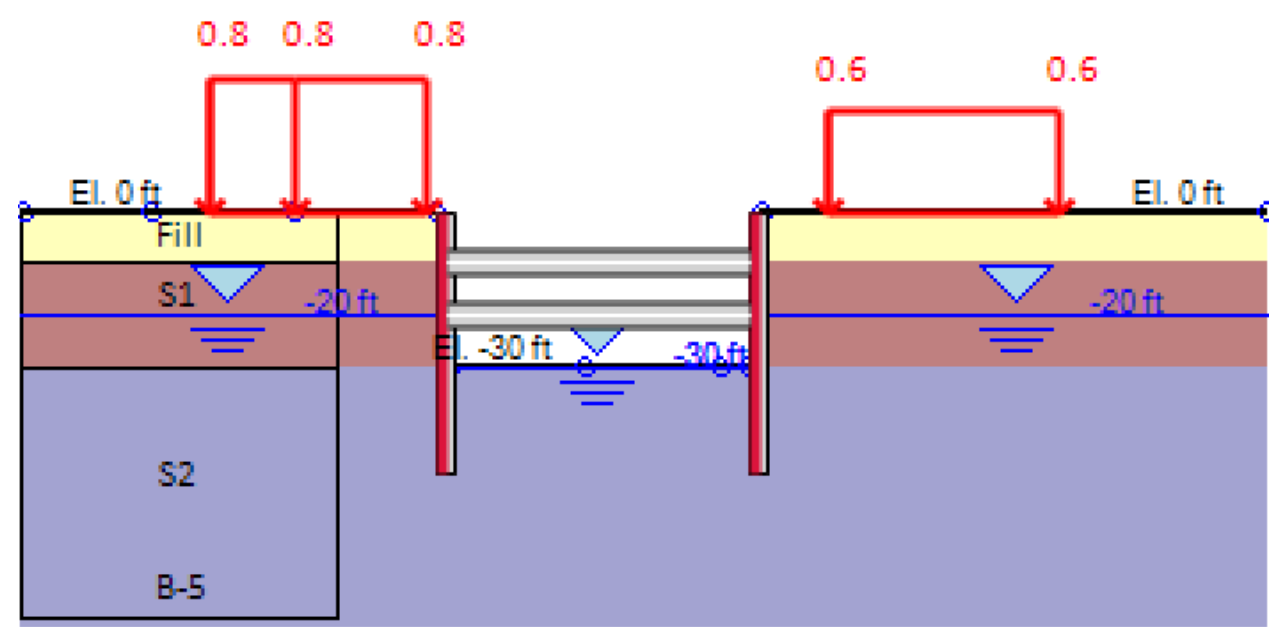
<p>Soldier pile and lagging walls</p> 	<p>Secant pile walls</p> 	<p>Tangent pile walls</p> 
<p>SPTC walls</p> 	<p>Diaphragm (slurry) walls</p> 	<p>Sheet pile walls</p> 
<p>Combined sheet pile walls</p> 	<p>Box sheet pile walls</p> 	<p>Custom walls</p> 



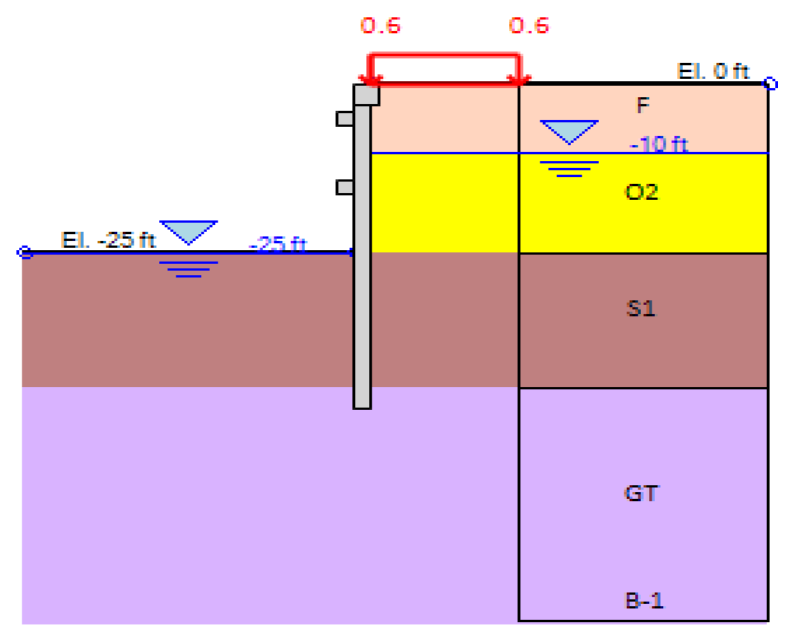
Anchored Walls (Tiebacks)



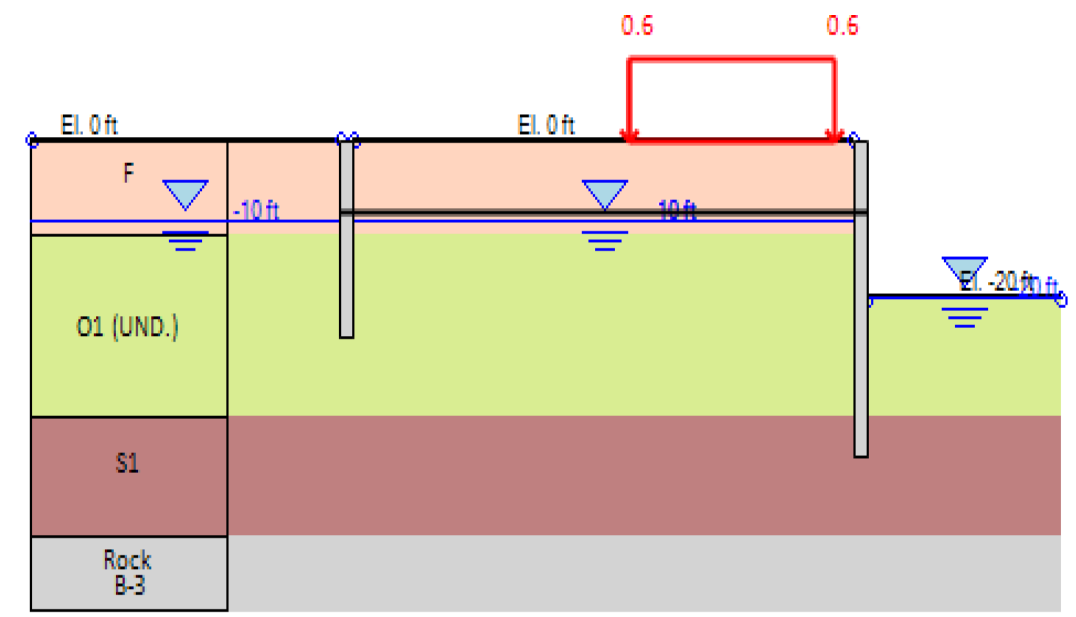
Braced Excavations (Struts and Rakers)



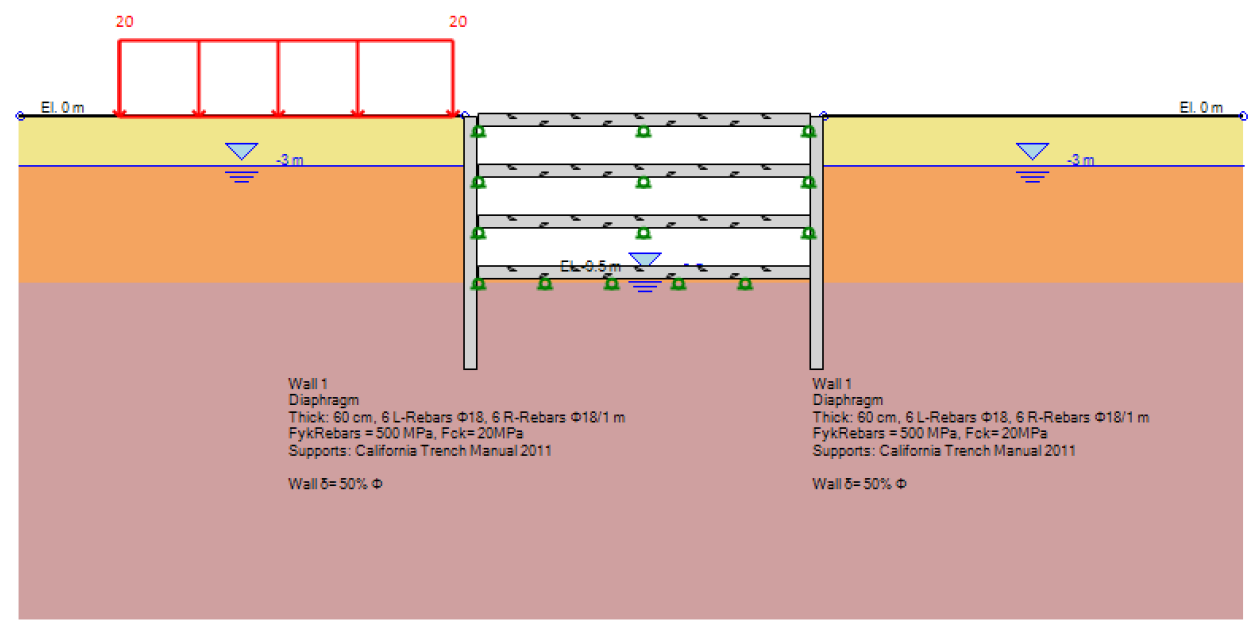
Circular Shafts (Ring Beams)



Dead-man Walls (Tierods)



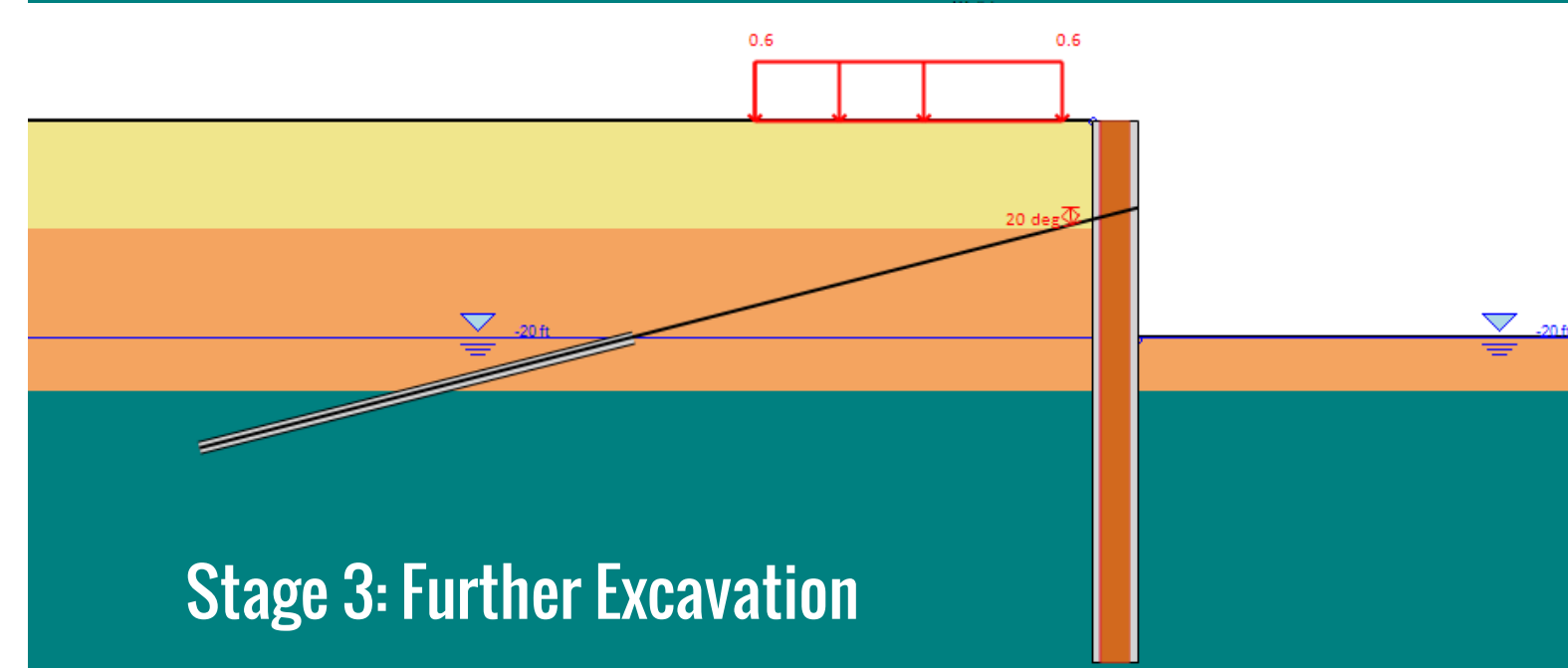
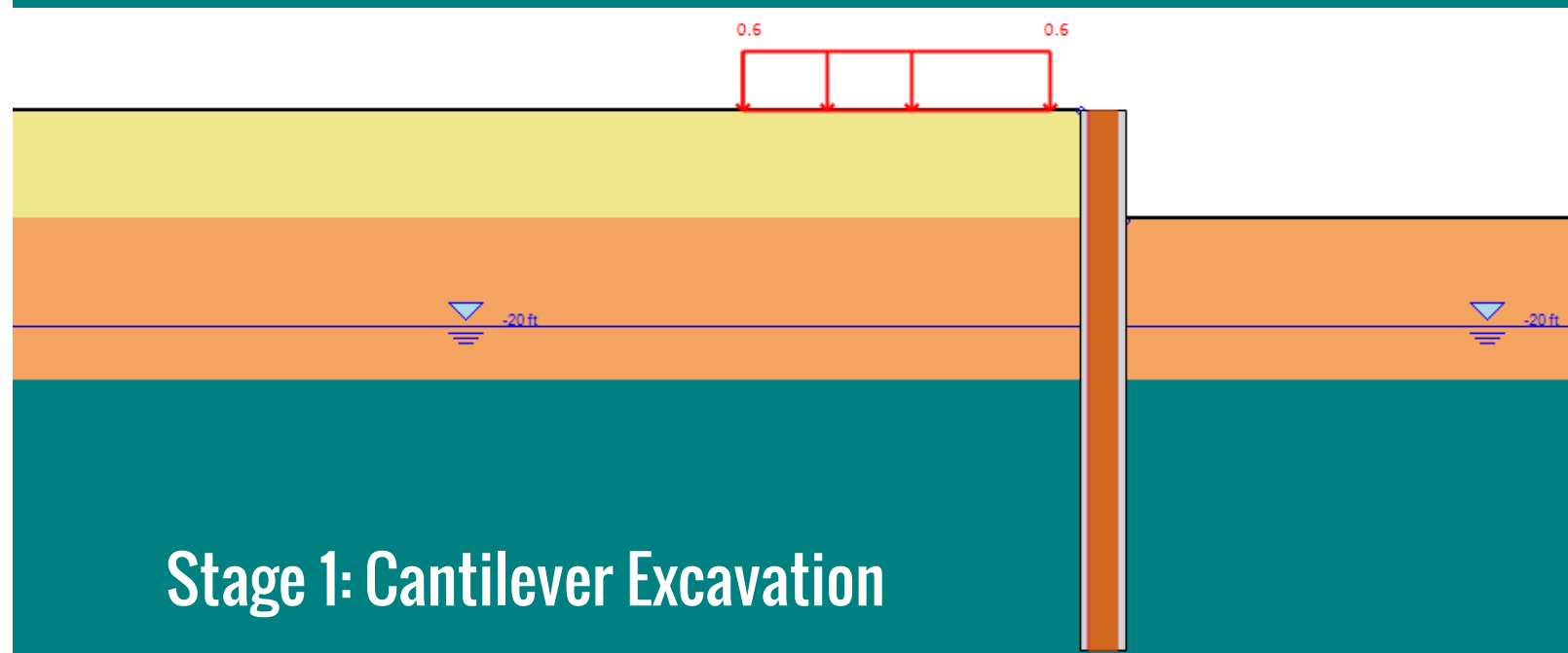
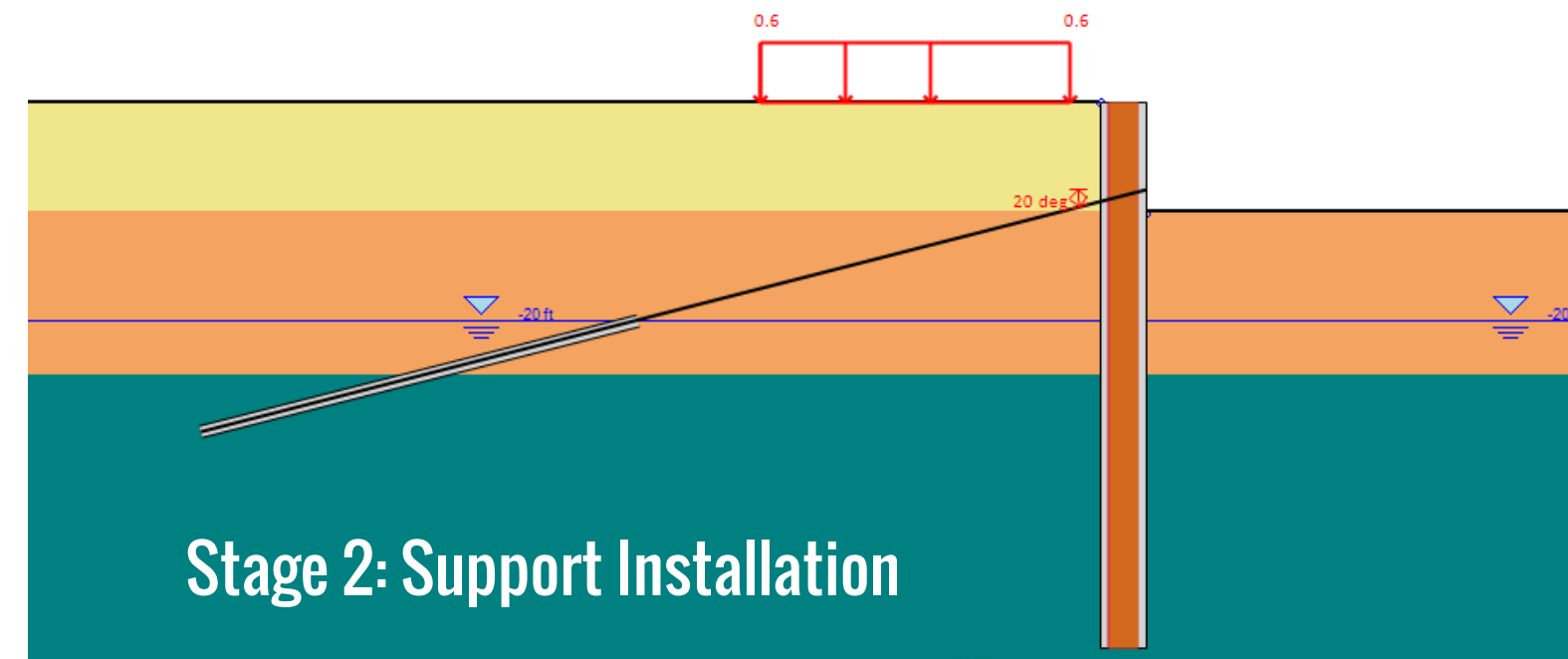
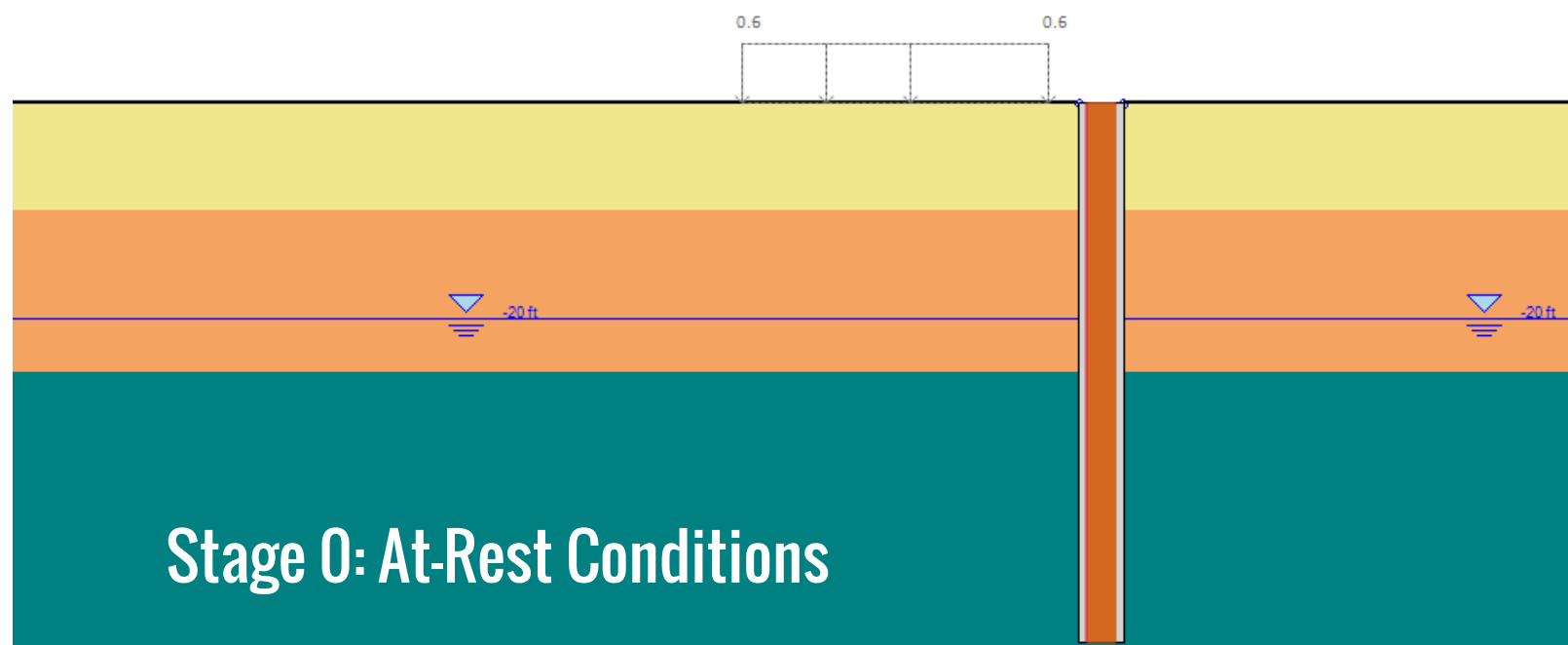
Top-Down Excavations (Concrete Slabs)



Include All Construction Stages

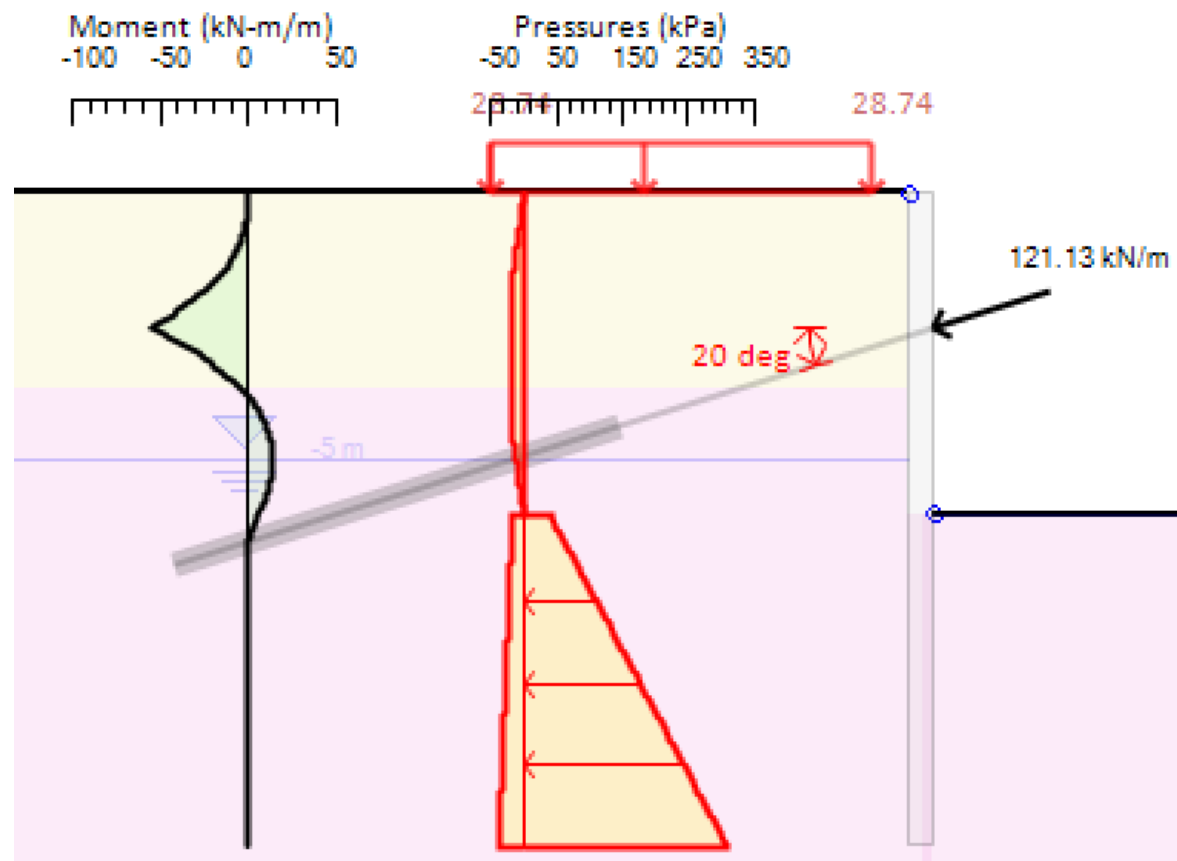


- ✓ Create all intermediate construction stages
- ✓ Review the results for each stage & recognize the critical stages
- ✓ Perform an efficient model optimization
- ✓ Get more realistic results for methods that consider staging (NL, FEM)

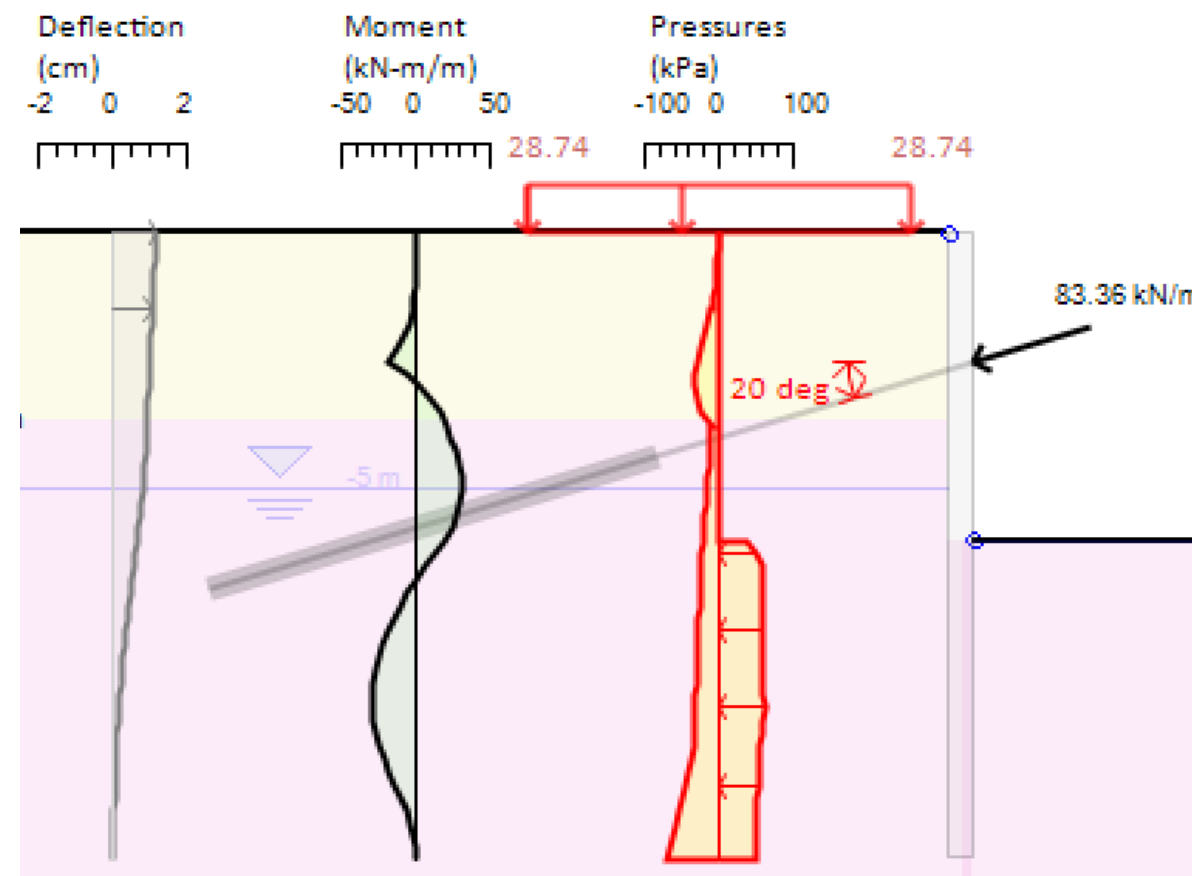




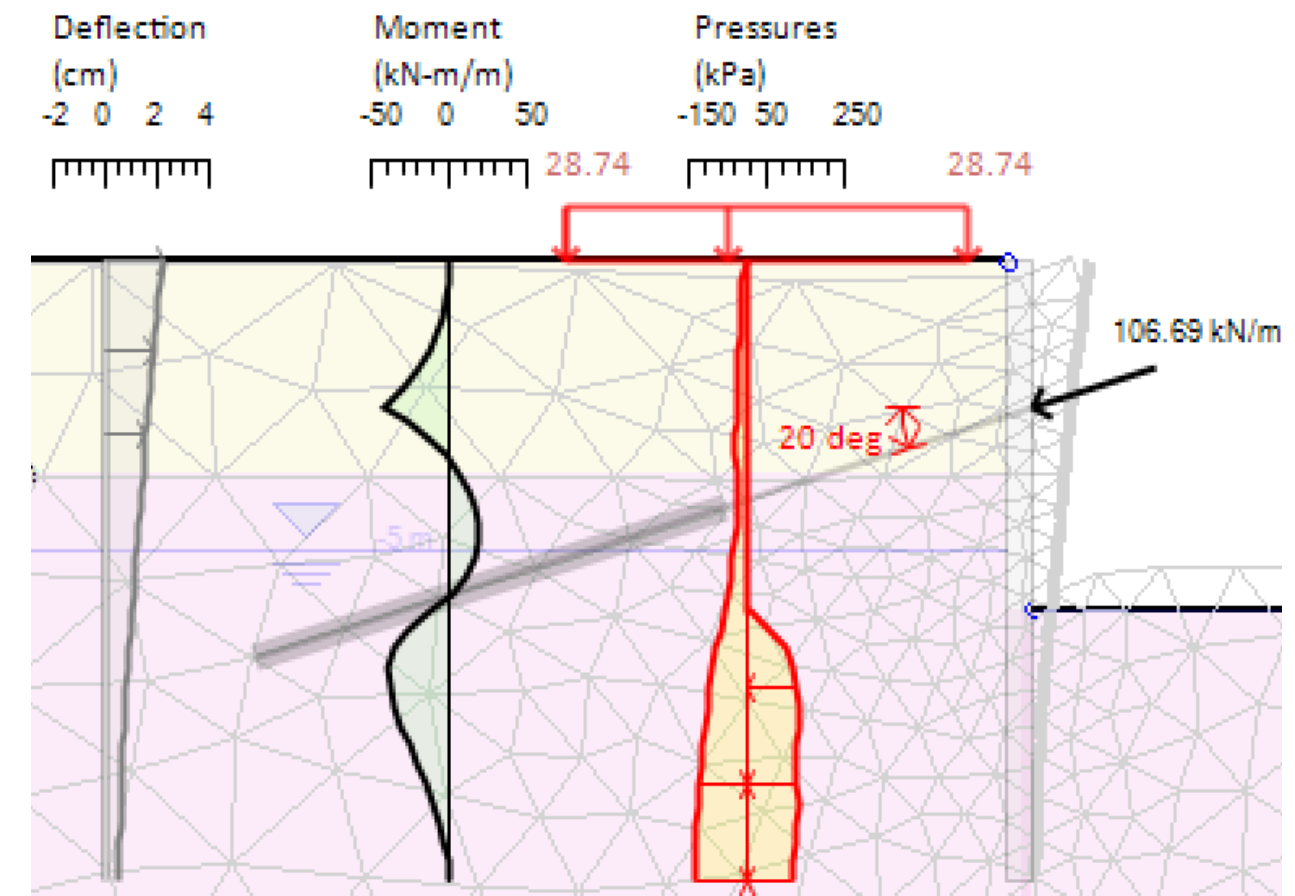
Limit Equilibrium Analysis (LEM)



Non-Linear Analysis (NL) (Elastoplastic Springs)

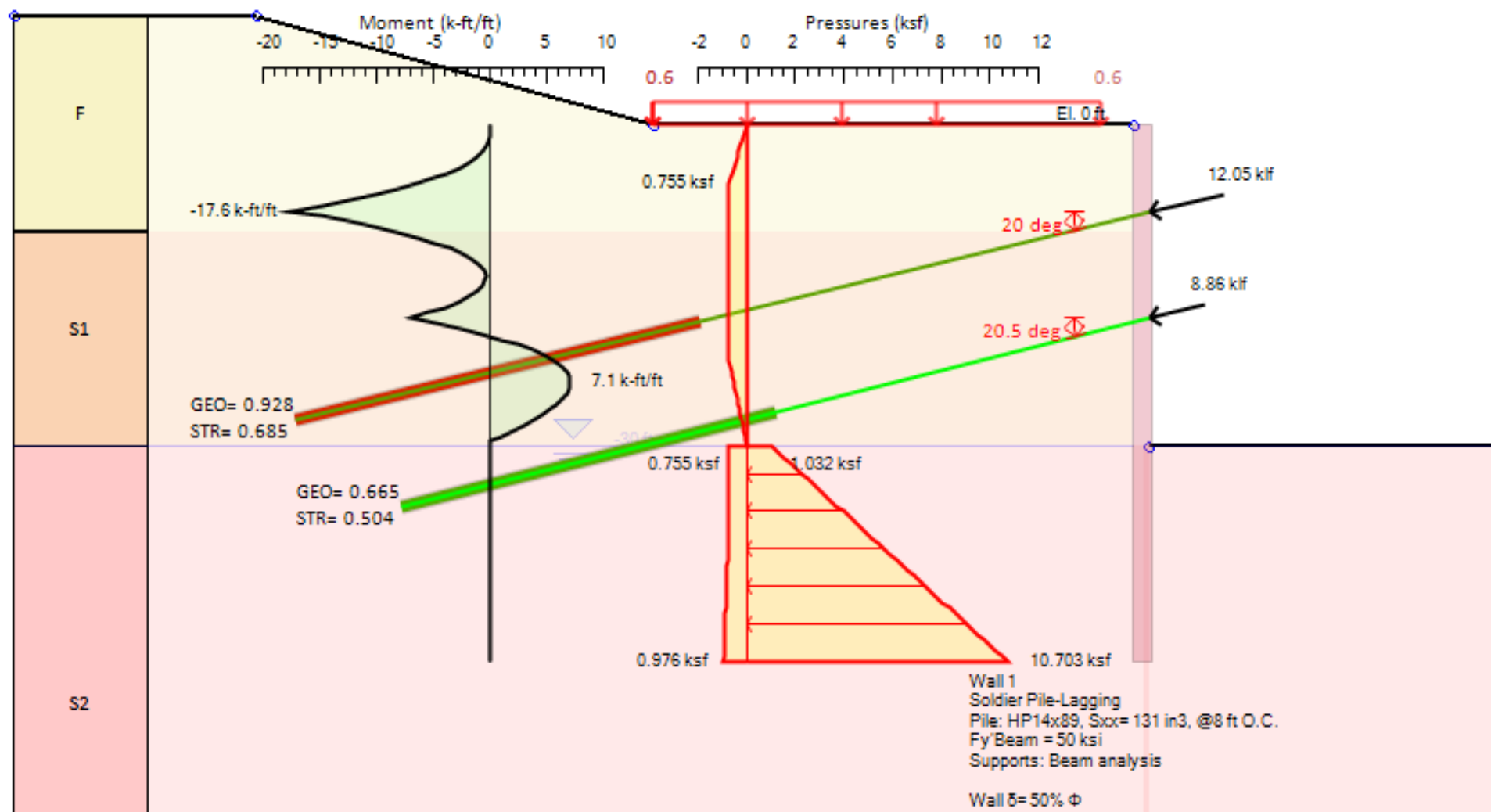


Finite Element Analysis (FEM)

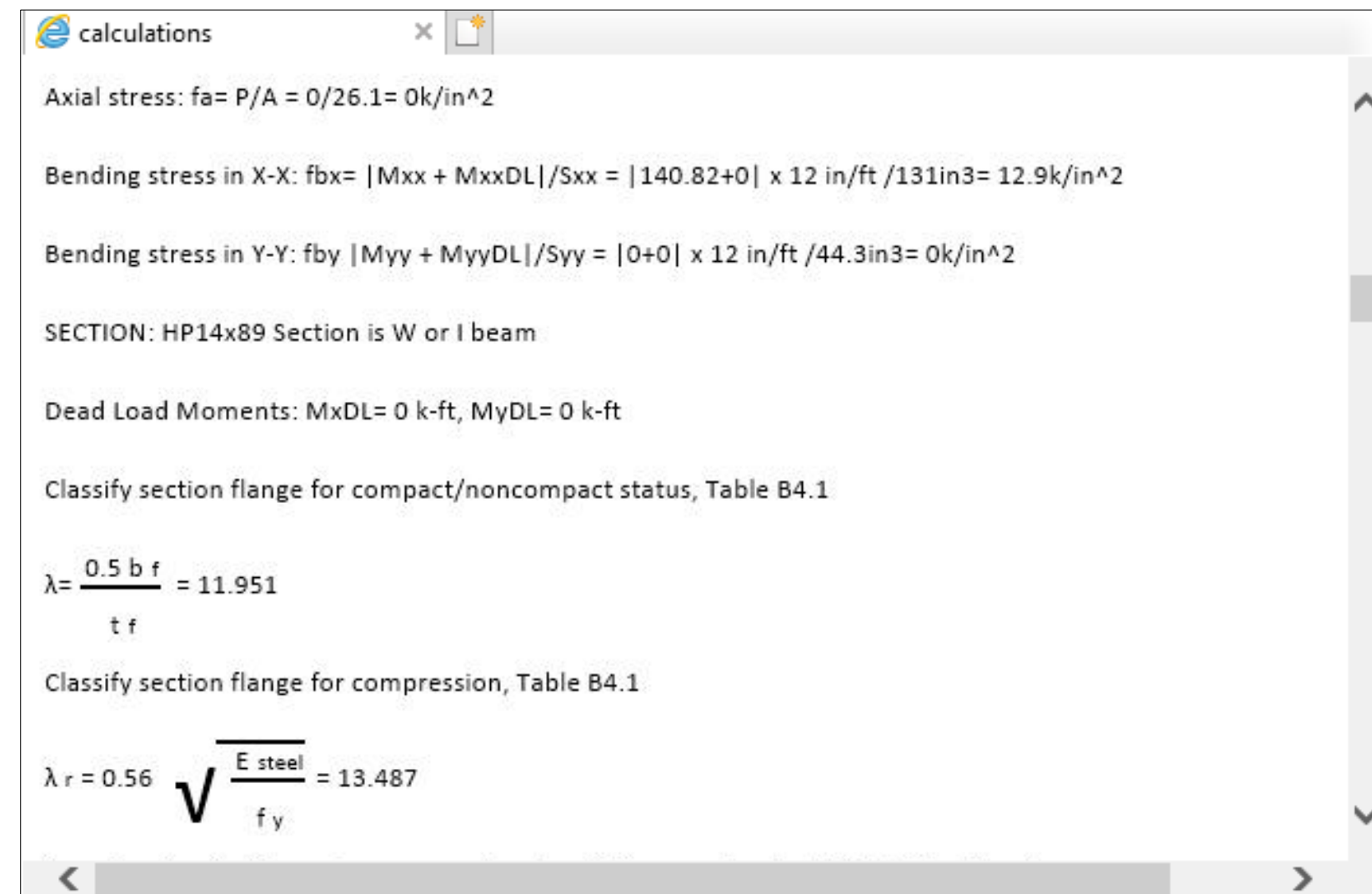




Diagrams, Reactions & Check Ratios:



Structural Checks & Design Calculations:

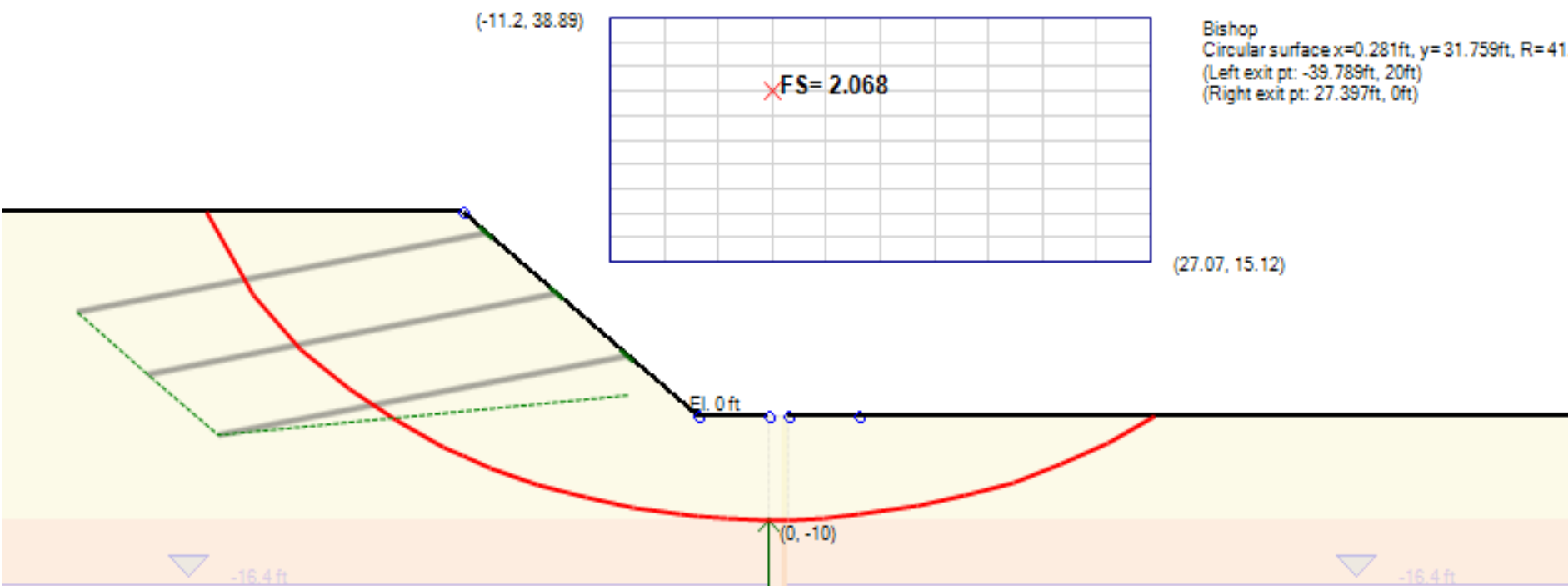


Structural Codes: Eurocodes 1,2 & 8, ACI, LRFD, AISC, AS 3600 & 4100, CN (China) + more

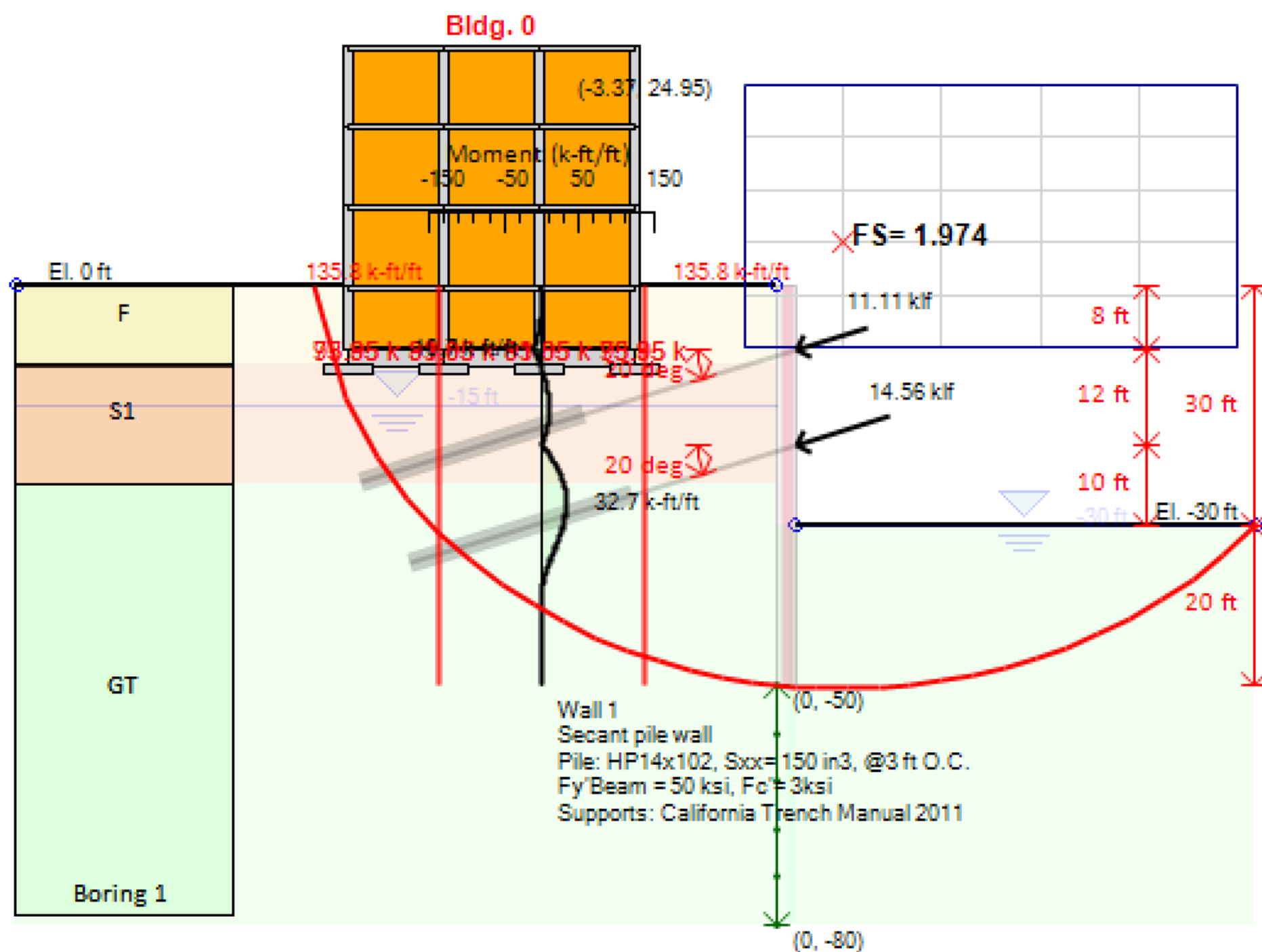
Design Standards: Eurocode 7, DIN, BS, XP, AASHTO LRFD, CALTRANS, CN (China) + more

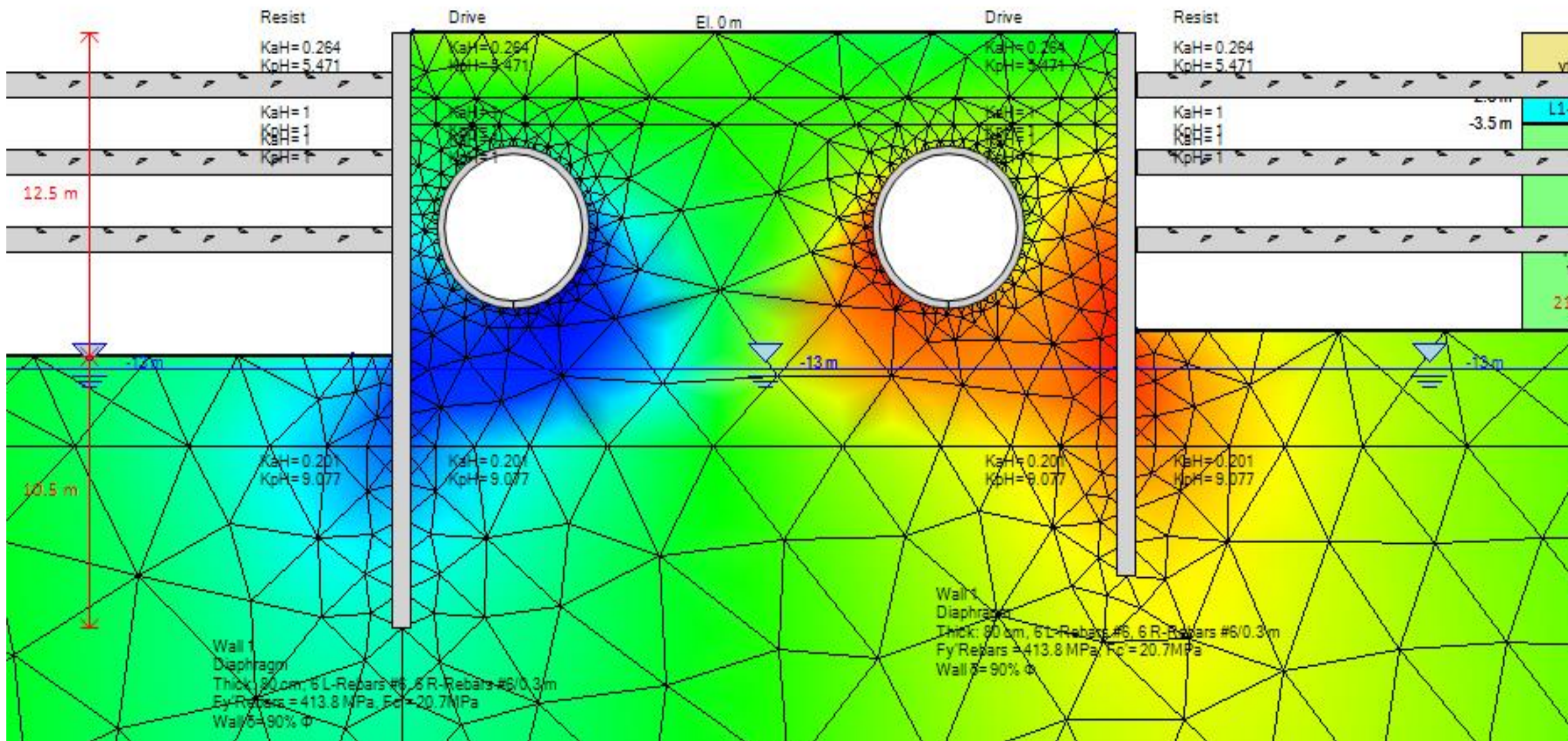


Slope Stability Analysis Options



- ✓ Bishop Method
- ✓ Morgenstern Price Method (G.L.E.)
- ✓ Spencer Method
- ✓ Ordinary (Swedish) Method
- ✓ Automatic Slope Search Method
- ✓ Single Point Slope Center
- ✓ Rectangular Slope Center
- ✓ Define Radius Search Limits
- ✓ Clouterre Standards for Soil Nails





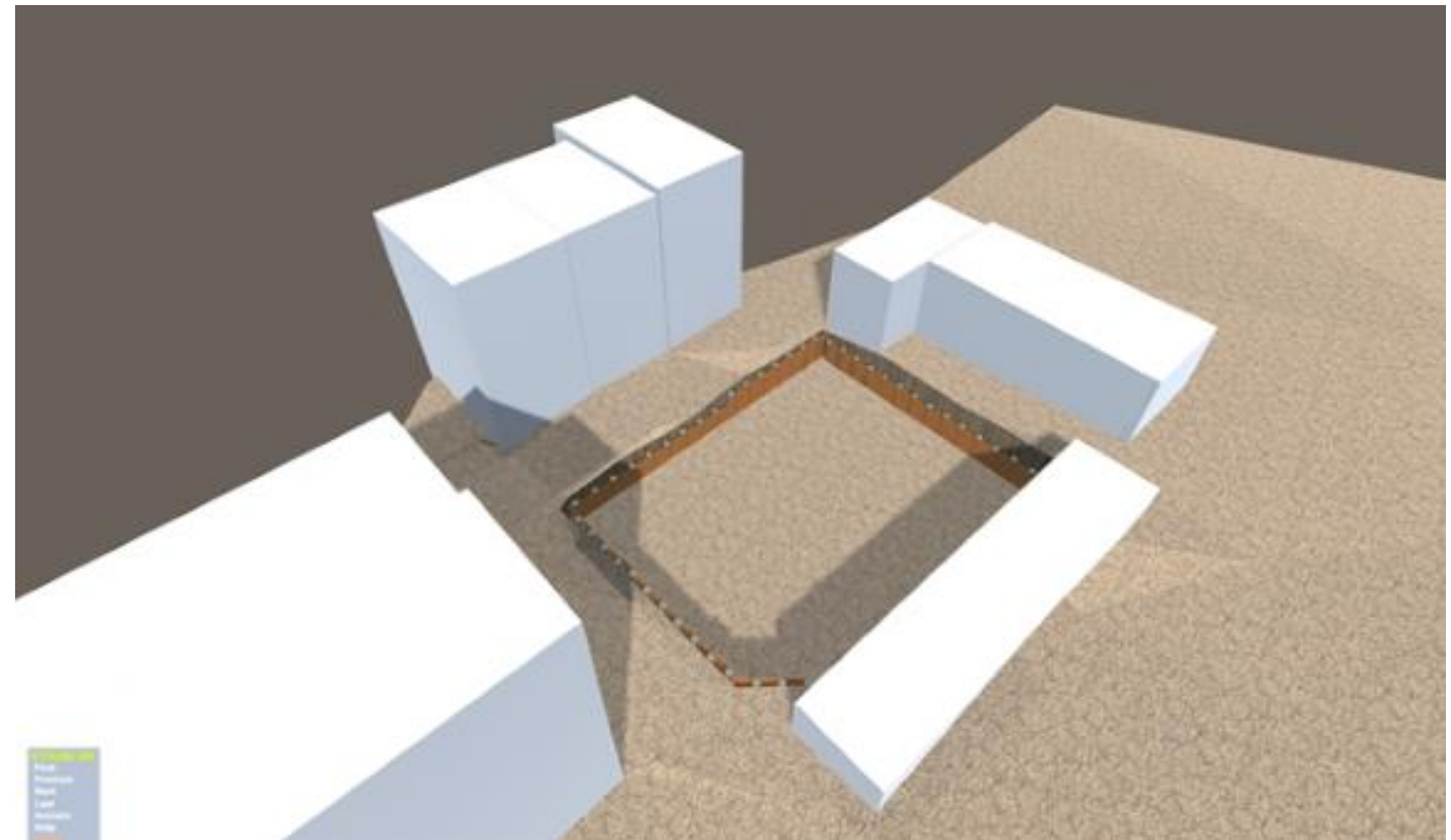
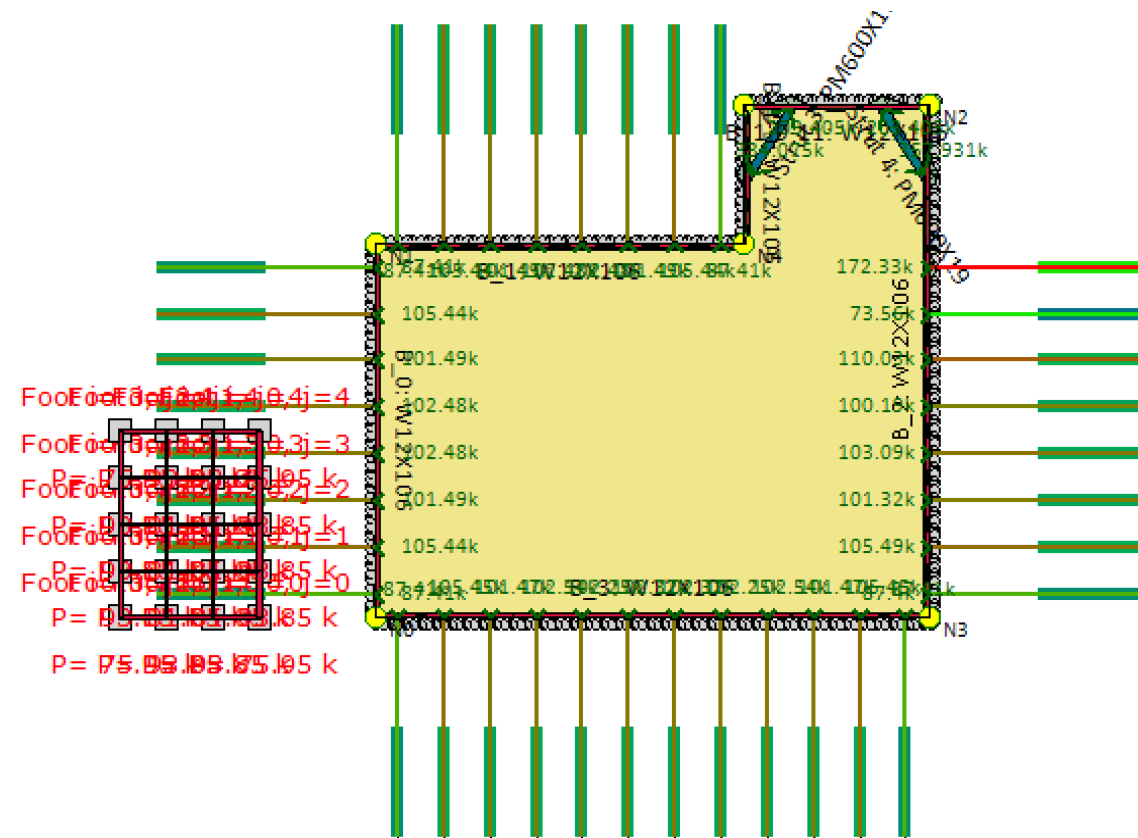
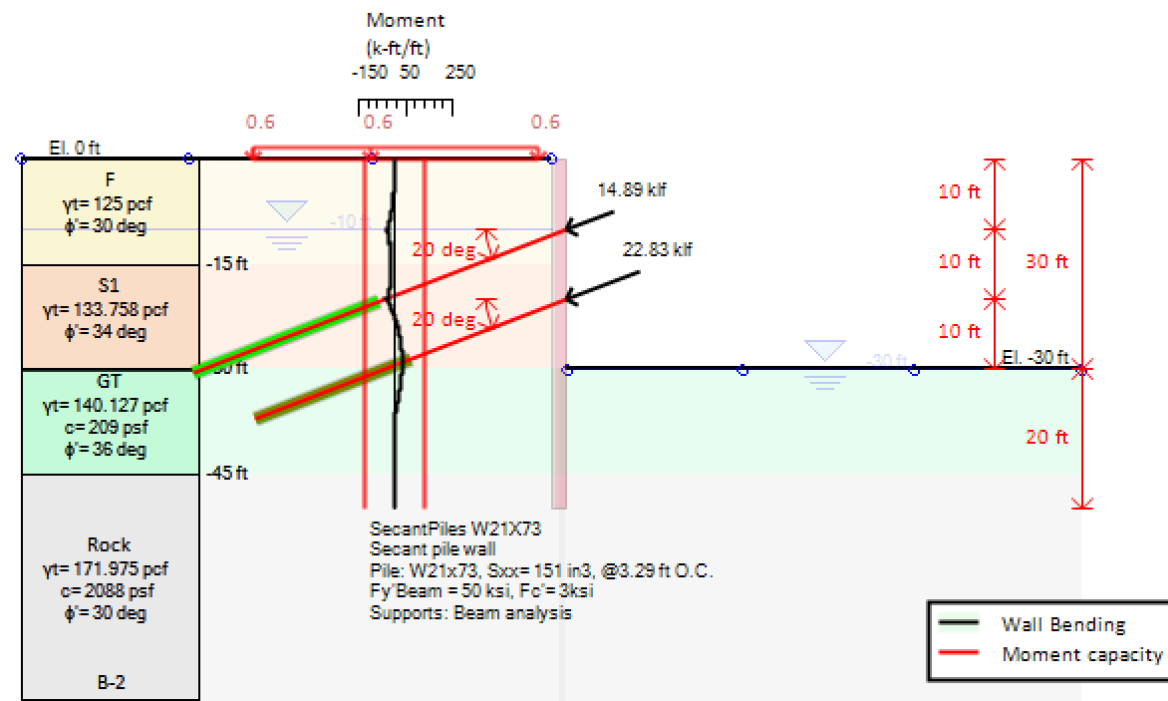
- ✓ DeepEX 2D FEM Engine (DeepFEM)
- ✓ Consider full soil-structure interaction
- ✓ Automatic FE options
- ✓ Soil Models for FEM
- ✓ Include Tiedowns & Foundation Piles

Tunnel Options:

- ✓ Tunnel Analysis with FEM
- ✓ TBM Tunnels
- ✓ NATM - SEM Tunnels
- ✓ Oval and Complex Tunnel Shapes
- ✓ Tunnel Model Wizard
- ✓ Cut-and-Cover Tunnels



- ✓ Full Design - 2D Sections and 3D Model
- ✓ Structural & Geotechnical design of Tiebacks and Struts
- ✓ 3D Building Loads
- ✓ Full Model Optimization (Walls and Supports)
- ✓ Virtual Reality Model Visualization - Export Model to HoloDeepEX





Steel Connection Data

Name and section type
Name: Stiffeners are not required
Horizontal angle: deg Max. weld stress check (all stages):

Input Stage Results

Connection Options
Weld Size: in Selected Welds:

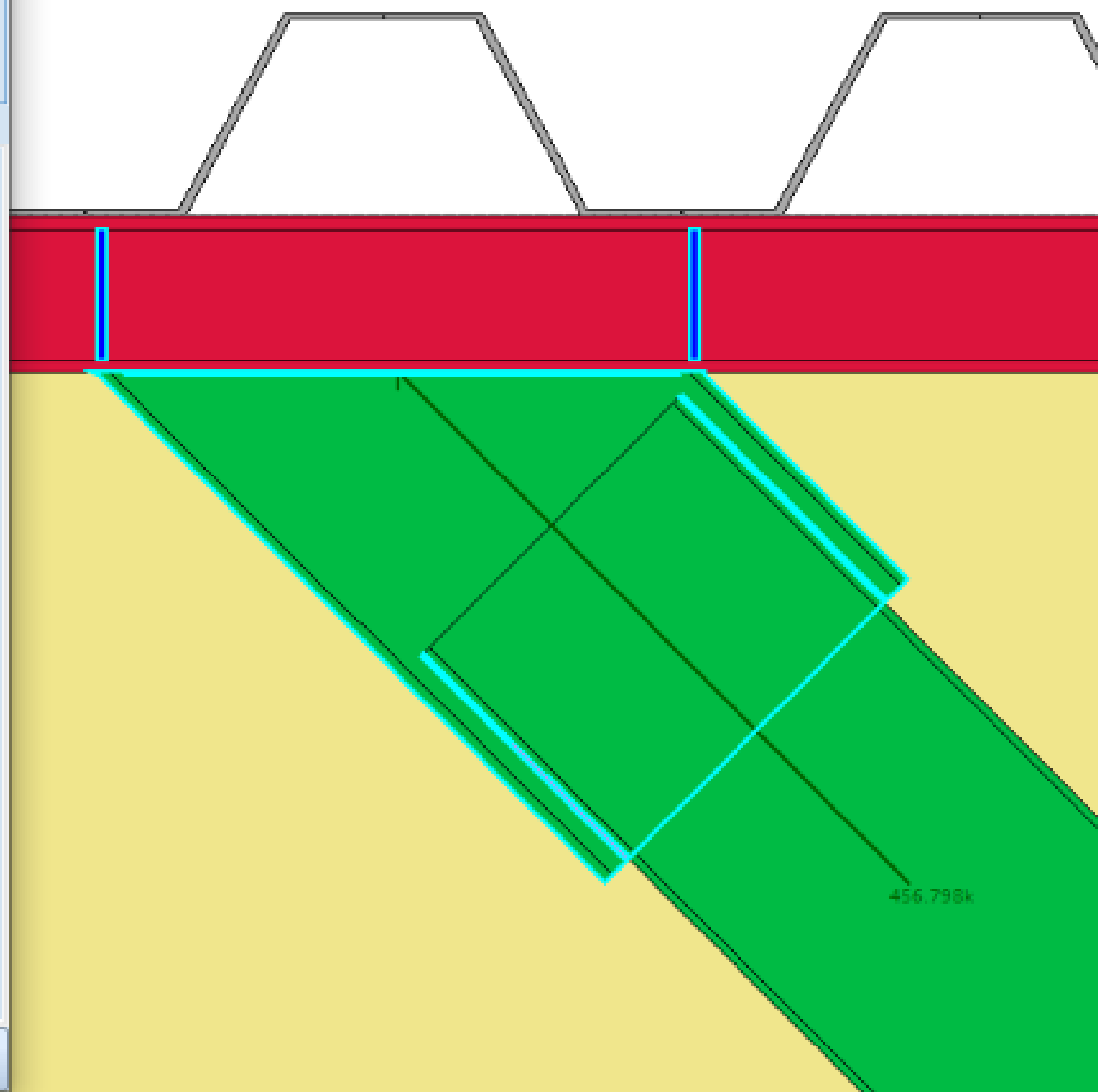
Connection Stub
Type: Use H (or W) beam stub
Stub section:
Min. overlap with strut: in
Clearance to strut: in
Weld (pipe to connector): in

Stiffeners

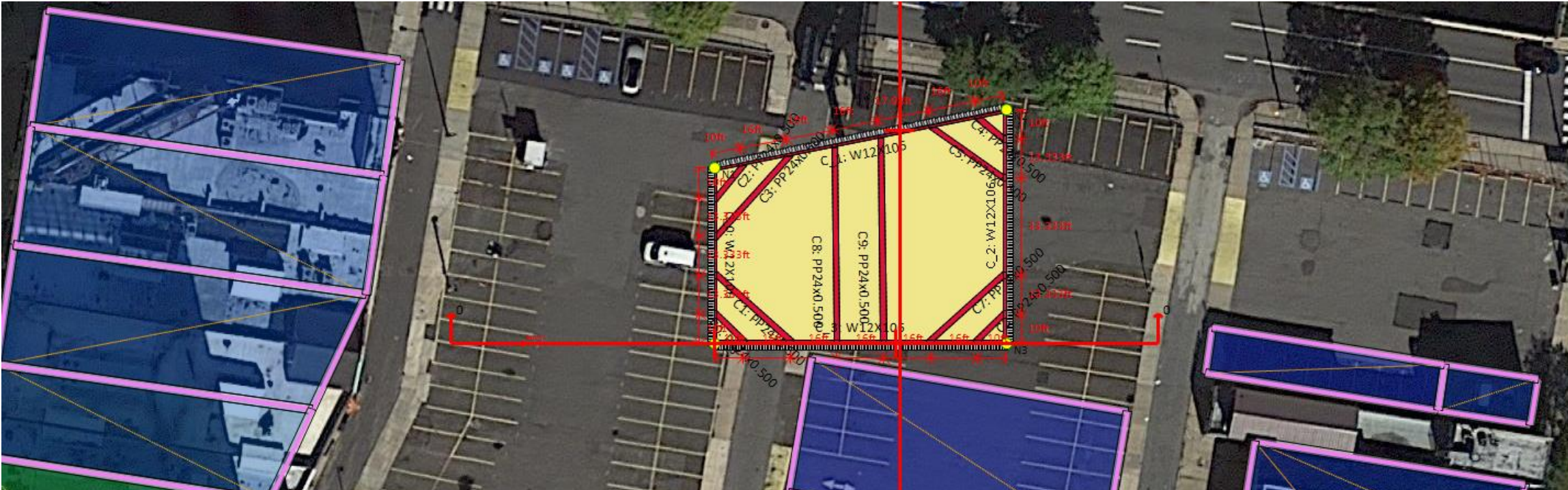
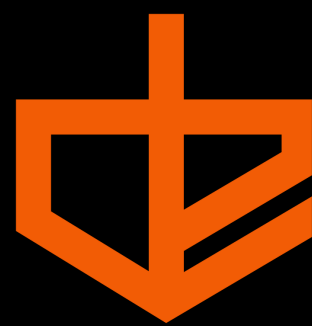
Stiffener Name	Location	Thick (in)	Height (in)	Width (in)
PL1_T	Top	0.75	5.7955	10.929
PL1_B	Bottom	0.75	5.7955	10.929
PL2_T	Top	0.75	5.7955	10.929
PL2_B	Bottom	0.75	5.7955	10.929

Weld Size: in

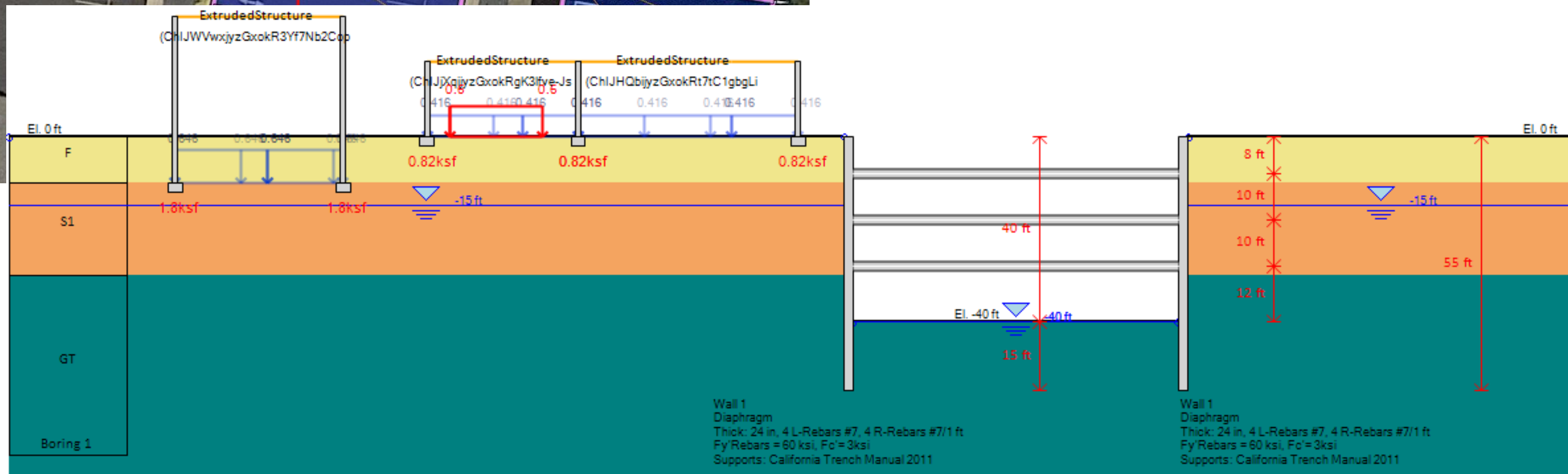
OK Cancel



- ✓ Generate all steel connections
- ✓ Check Steel Connections (Struts and Walers)
- ✓ Optimize Steel Connections with a Click
- ✓ Adjust weld sizes and apply plate stiffeners



- ✓ Import your Excavation Site from Google Maps
- ✓ Import all buildings directly from Google
- ✓ Estimate the building dimensions and loads
- ✓ Generate 2D cut sections
- ✓ Perform Damage Assessment for all buildings

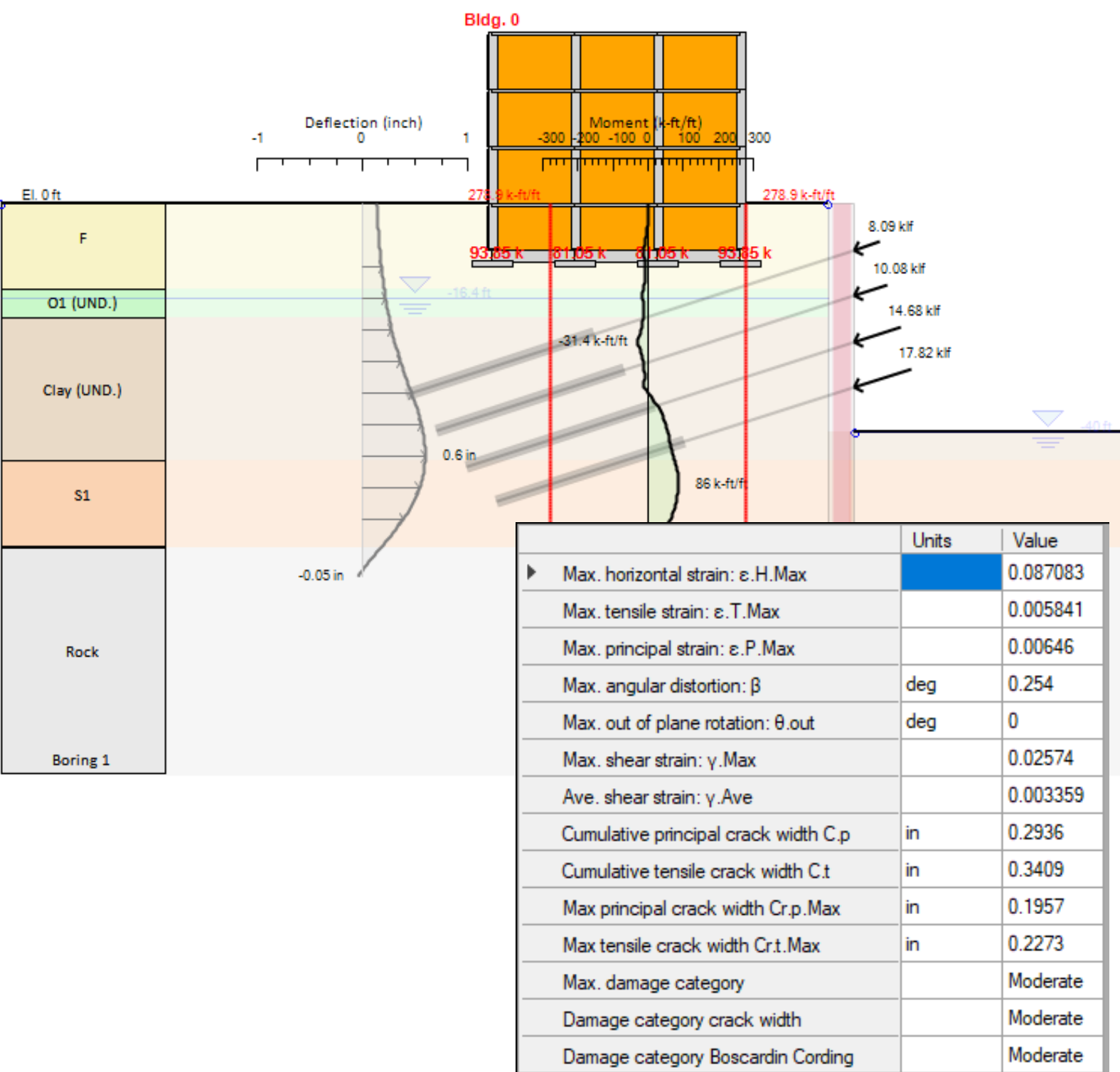




Building Damage Assessment



- ✓ Perform Damage Assessment of all Buildings close to an excavation site
- ✓ Review Crack widths, Damage Categories, Strains etc. for all building walls.

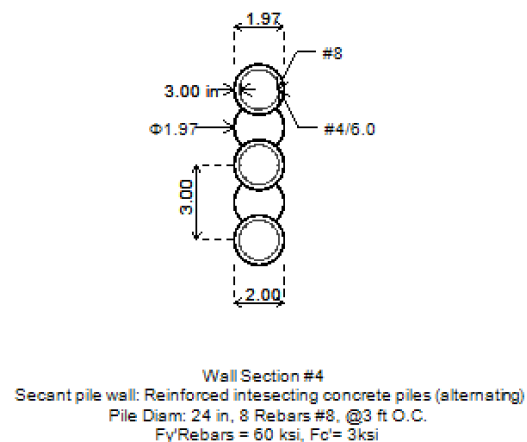
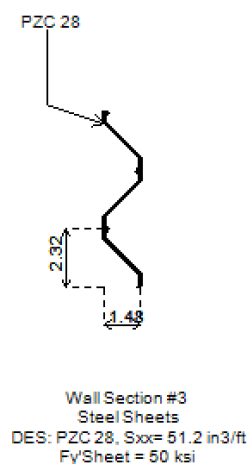
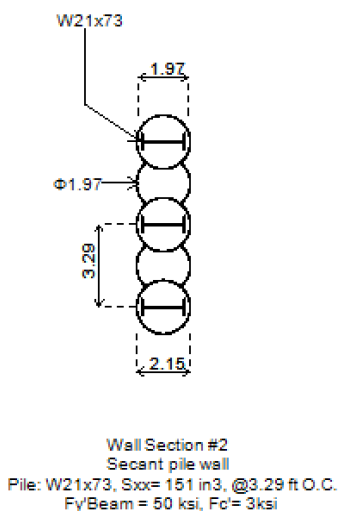
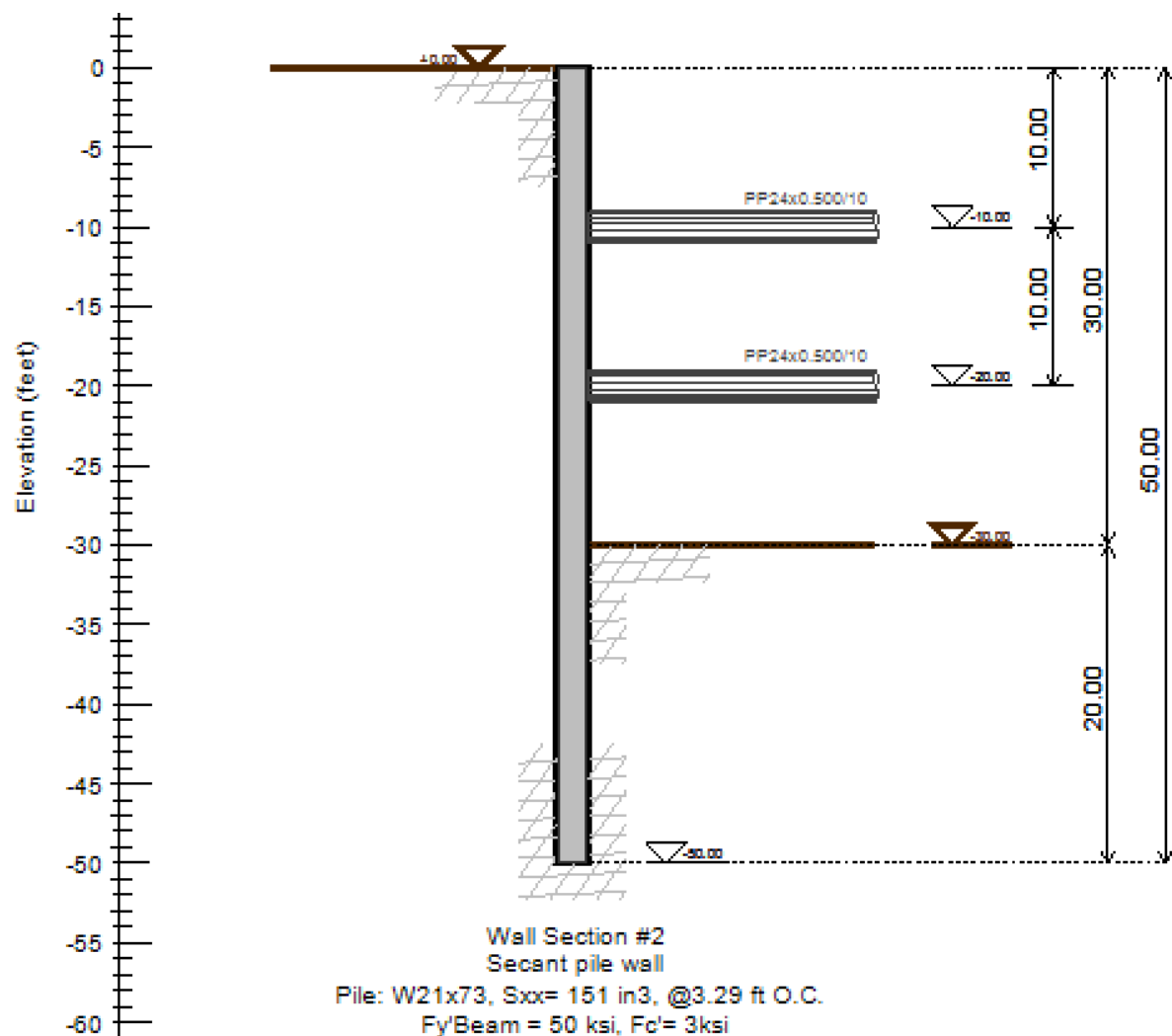


Building Damage Results

Bldg. 0

Maximum values | All elements | Individual elements | Horizontal movement | Settlement | Boscardin-Cording Chart | Hogging Chart (Burland 1979)

		$\theta.out$	$\gamma.Max$	$\gamma.Ave$	C.p (in)	C.t (in)	Cr.p (in)	Cr.t (in)	Damage Cat	Dam. Crack width	Dam. Boscardin
Bottom side continuous basement wall	203	0	0.005103	0.002551	0	0	0	0	Moderate	Negligibe	Moderate
Left side continuous basement wall	277	0	0.000752	0.000376	0	0	0	0	Negligibe	Negligibe	Negligibe
Right side continuous basement wall	926	0	0.002511	0.001256	0	0	0	0	Negligibe	Negligibe	Negligibe
Top side continuous basement wall		0	0	0	0	0	0	0	Negligibe	Negligibe	Negligibe
Exterior wall at floor 1El. 0, (-10.67, 30 to -20.67,30)	407	0	0.025554	0.001561	0.2327	0.3337	0.1551	0.2225	Moderate	Moderate	N/A
Exterior wall at floor 1El. 0, (-20.67, 70 to -10.67,70)		0	0	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-20.67, 30 to -30.67,30)	872	0	0.022453	0.002095	0.0798	0.0299	0.0532	0.0199	Slight	Slight	N/A
Exterior wall at floor 1El. 0, (-30.67, 70 to -20.67,70)		0	0	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-40.67, 30 to -30.67,30)	558	0	0.014831	0.001949	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-40.67, 70 to -30.67,70)		0	0	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-10.67, 40 to -10.67,30)	439	0	0.02574	0	0.153	0.2154	0.102	0.1436	Slight	Slight	N/A
Exterior wall at floor 1El. 0, (-40.67, 30 to -40.67,40)	34	0	0.007772	0.001901	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-10.67, 50 to -10.67,40)	169	0	0.02417	0	0.2936	0.1803	0.1957	0.1202	Slight	Slight	N/A
Exterior wall at floor 1El. 0, (-40.67, 40 to -40.67,50)	391	0	0.002266	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-10.67, 60 to -10.67,50)	336	0	0.013545	0	0.0476	0	0.0317	0	Very slight	Very slight	N/A
Exterior wall at floor 1El. 0, (-40.67, 50 to -40.67,60)		0	0	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-10.67, 70 to -10.67,60)		0	0	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 1El. 0, (-40.67, 60 to -40.67,70)		0	0	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 2El. 10, (-10.67, 30 to -20.67,30)	407	0	0.025554	0.001561	0.1975	0.2805	0.1317	0.187	Slight	Slight	N/A
Exterior wall at floor 2El. 10, (-20.67, 70 to -10.67,70)		0	0	0	0	0	0	0	Negligibe	Negligibe	N/A
Exterior wall at floor 2El. 10, (-20.67, 30 to -30.67,30)	872	0	0.022453	0.002095	0.1566	0.2011	0.1044	0.1341	Slight	Slight	N/A

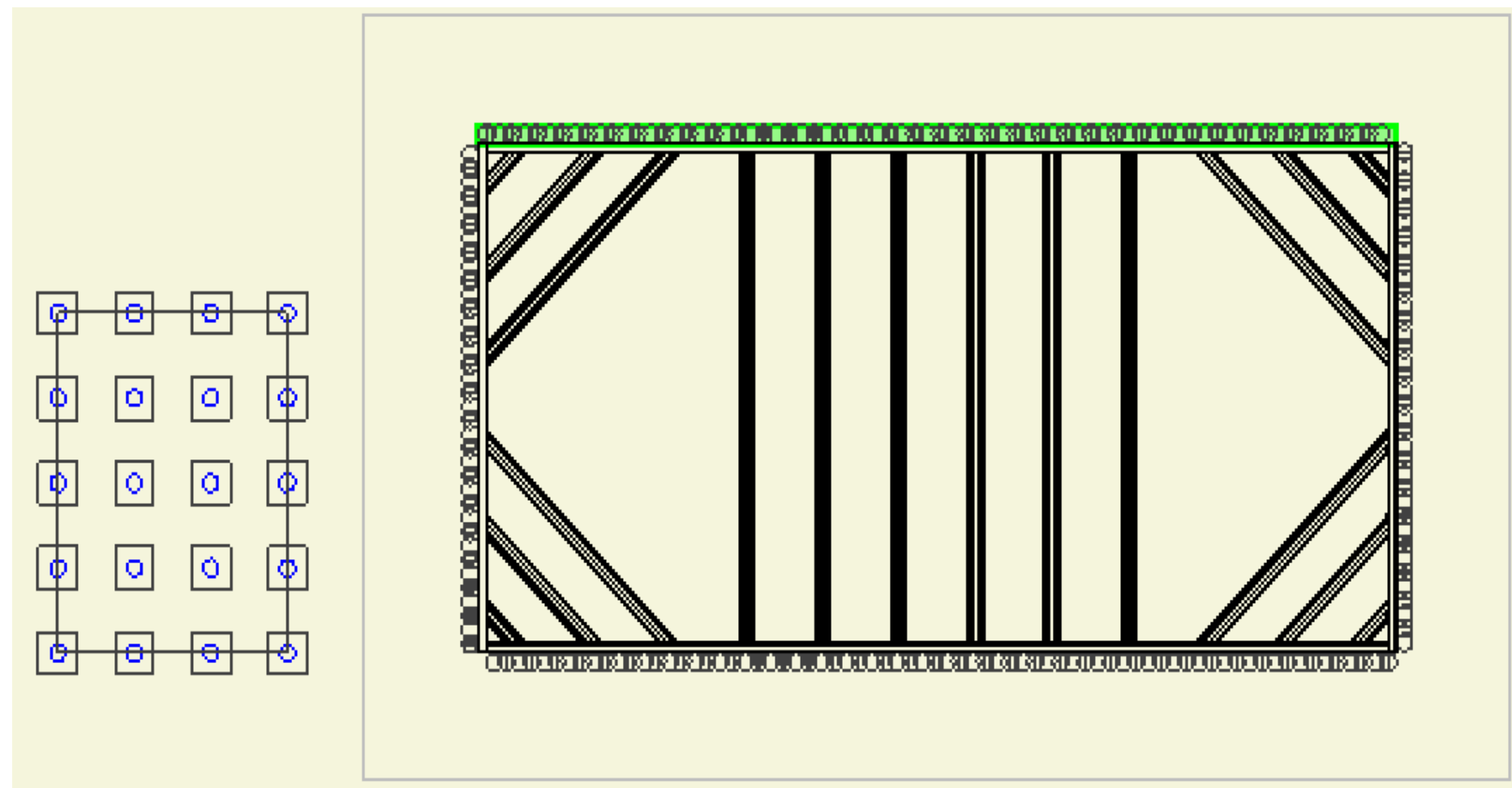


2D Sections:

- ✓ Export all 2D Sections Sketches for each Construction Stage
- ✓ Export Wall Section Details
- ✓ Export 2D Sections with Result Diagrams

3D Models:

- ✓ Export all 2D Sections and Wall Details
- ✓ Export Full Project Plan Sketches
- ✓ Export Elevation Sketches for each Project Wall

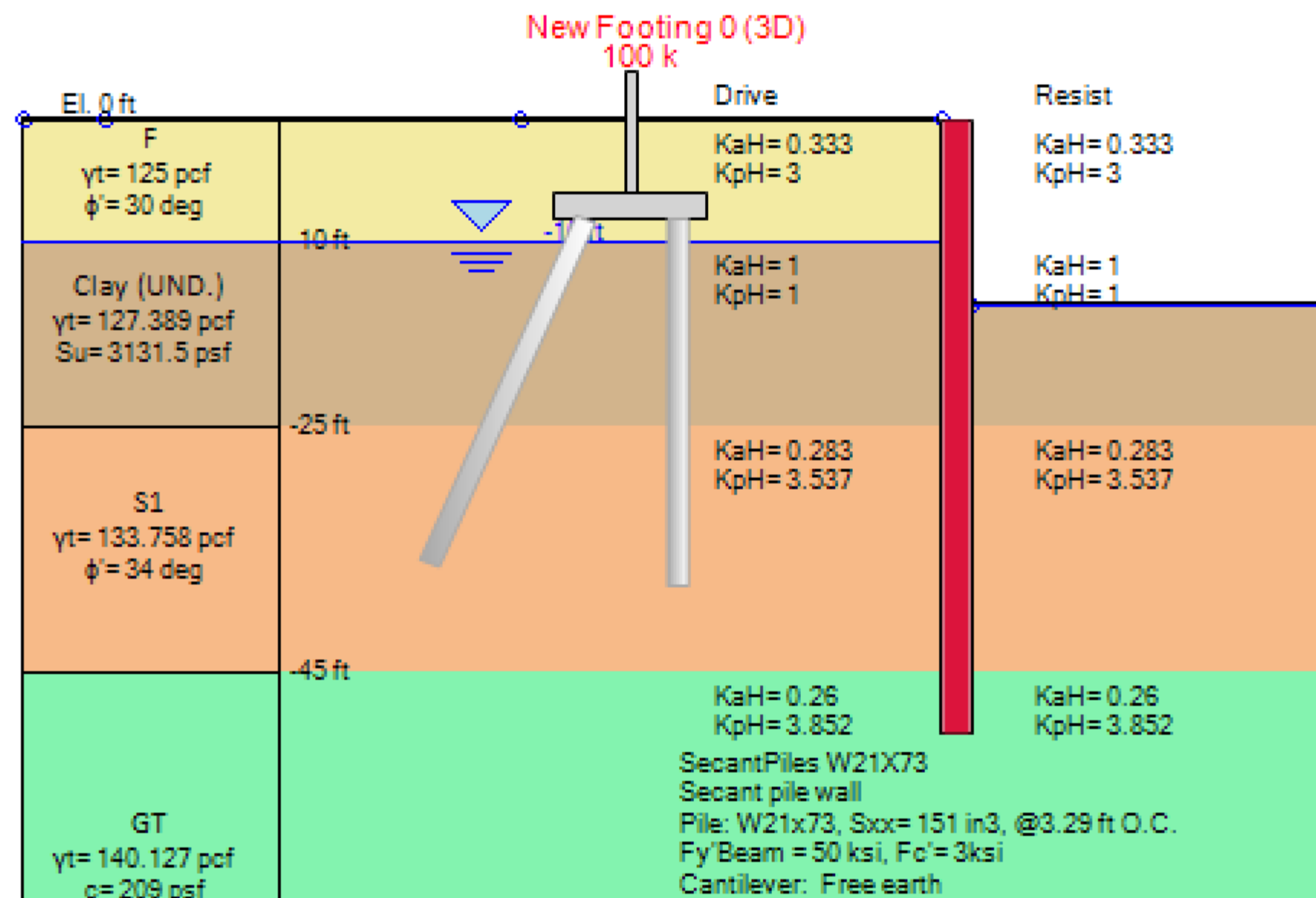
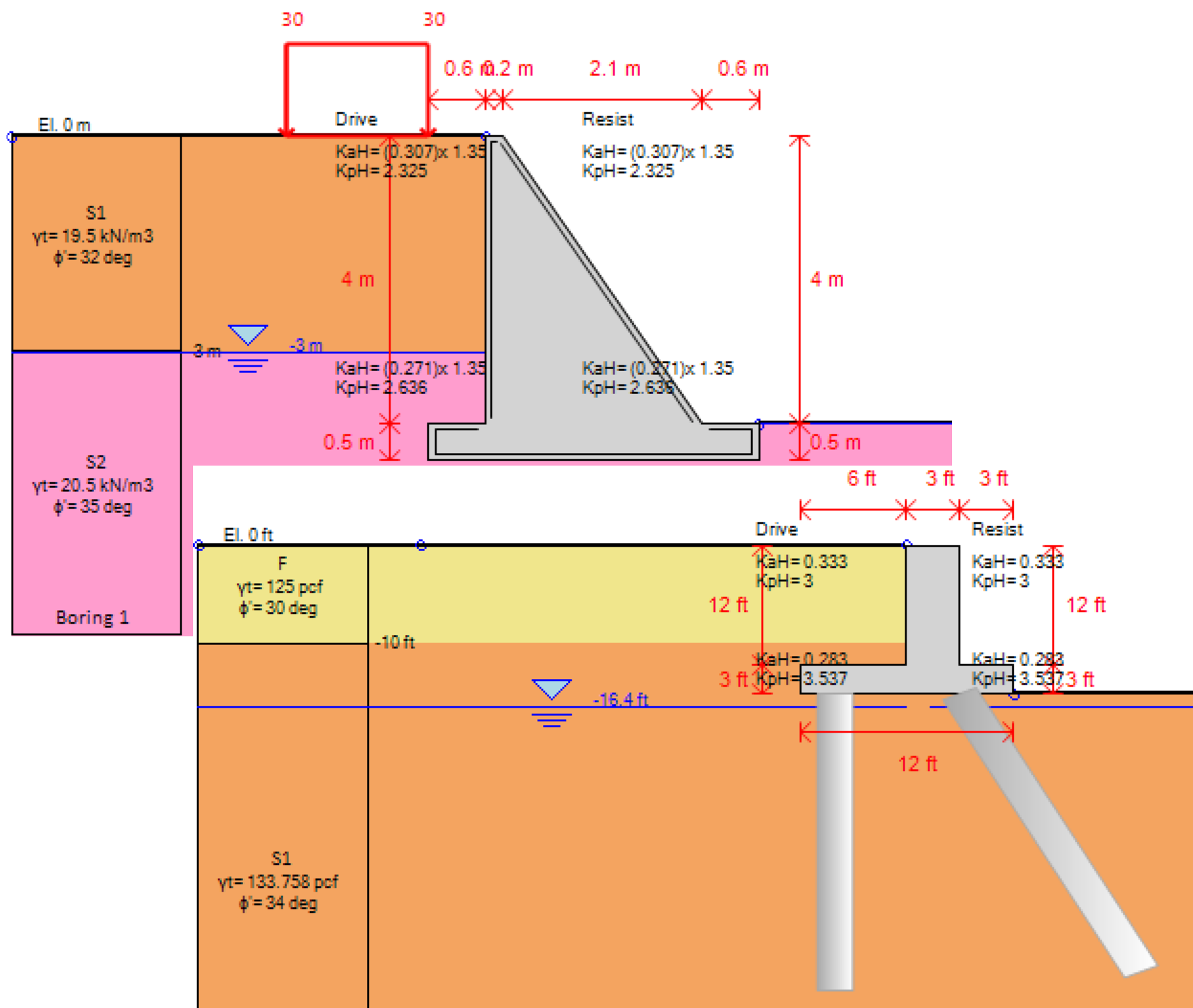




Gravity Walls & Pile Abutments

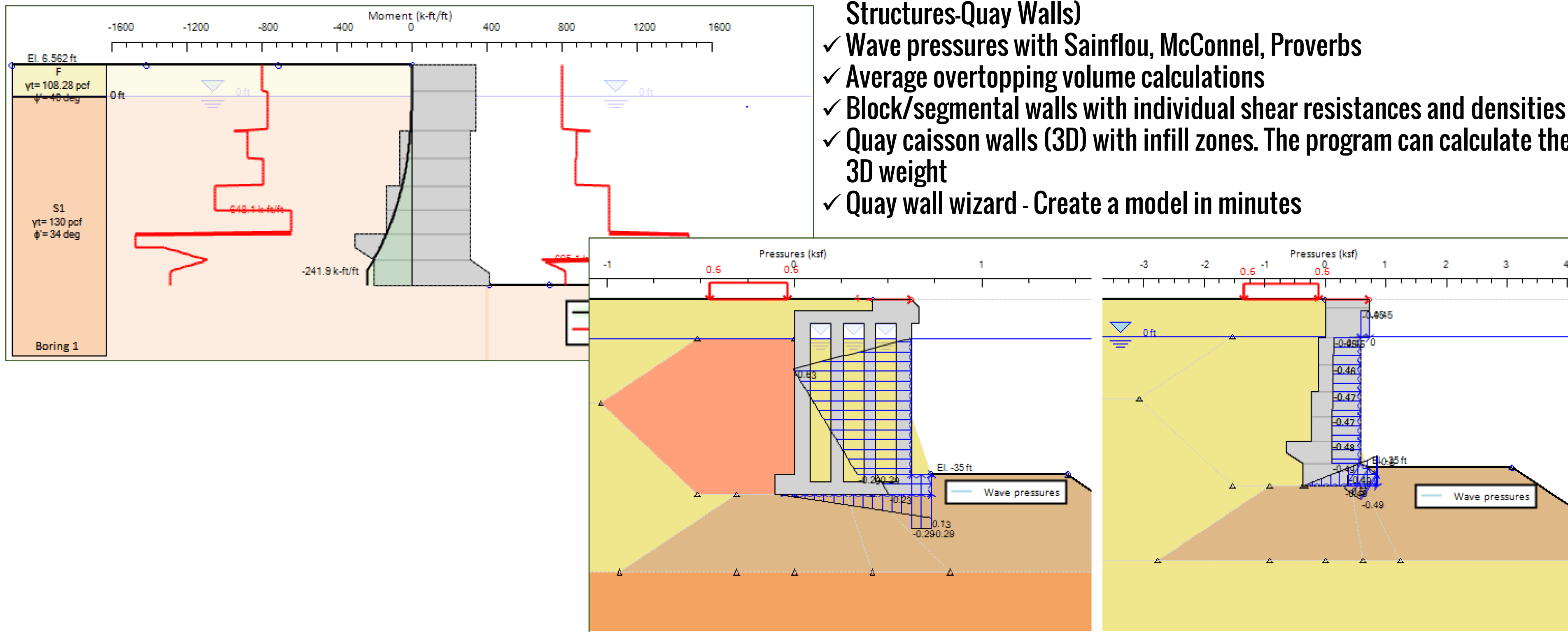


- ✓ Design gravity walls (any shape)
- ✓ Design pile supported abutments
- ✓ Use footings (3D loads) and design the foundation piles





- ✓ Load combinations for British Standards 6349 Parts 1-2 (Marine Structures-Quay Walls)
- ✓ Wave pressures with Sainflou, McConnell, Proverbs
- ✓ Average overtopping volume calculations
- ✓ Block/segmental walls with individual shear resistances and densities
- ✓ Quay caisson walls (3D) with infill zones. The program can calculate the 3D weight
- ✓ Quay wall wizard - Create a model in minutes





- ✓ Estimate Soil Properties with different methods
- ✓ Review a statistical analysis of the estimated properties
- ✓ Select the project values with a high level of certainty

Depth	SPT	RQD (%)
1	4	0
6	5	0
11	11	0
16	11	0
21	16	0
26	13	0
31	18	0
36	18	0
41	14	0
46	19	0

1. Name and material

Set 1 Determine confidence values at Lower bound 25 %

2. Density and Strength 3. Elasticity 4. Bond Resistances 5. Lateral Pile 6. OCR

Select Equations to use for estimating soil parameters

2.A: Soil Density

- γ Kullhawy, Mayne, 1990, Table 2-9, pg. 1-54
- DR, Bowles et. al., DeepEX approach
- DR, Manual of Estimating Soil Parameters, Table 2-9, pg. 2-19

2. B: Effective Friction Angle

- Φ Parry, 1977 (Perko, Helical Pile Design Manual)
- Φ triaxial compression calibration, FHWA NHI 132031
- Φ Kullhawy, Chen, 2007
- Φ Terzaghi & Peck, 1967
- Φ FHWA pilot database calibrations
- Φ_{cv} , Parry 1977 for clays
- Φ Kullhawy, Mayne, 1990
- Φ Sabatini et. al, 2002, FHWA NHI-10-106
- Φ_{cv} , Holtz-Kovac 1991, 1985 for clays vs. PI, lower bound
- Φ_{cv} , Holtz-Kovac 1991, 1985 for clays vs. PI, average
- Φ_{cv} , Holtz-Kovac 1991, 1985 for clays vs. PI, upper bound

2. C: Undrained Shear Strength

- $S_u = 0.06 N Pa = 0.125 N (ksf)$, Kullhawy, Mayne, 1990, Eq 4-59, p
- $S_u (ksf) = 0.13 N$, Terzaghi-Peck 1967
- S_u vs OCR, Ladd 1977, Jamiolkowski 1985
- S_u clays, Koutsoftas & Ladd, 1985, vs. OCR and PI

1. Select Set

Set 1

2. Result Type

- γ estimate
- γ estimate
- Φ estimate
- S_u estimate
- E estimate
- qBond estimate

4. Summary Results

Parameter estimation for:

Density: γ

Soil type: F

Sample count: 13

Average Input values

Average Nspt= 15.31 bpf

Average Relative density DR= 40.52 %

Estimate results

Average estimate $\gamma = 115.138$ pcf

Standard deviation $\gamma = 4.266$ pcf

Max. value $\gamma_{max} = 120.9$ pcf

Min. value $\gamma_{min} = 109.4$ pcf

Confidence level $\gamma_{des} = 115.138$ pcf

Confidence level at 50% lower bound

5. Adjust or Pass to soil type

Lower bound 50 %

Determine new design value based on lower bound percentage

Pass value to soil type

Preview Report

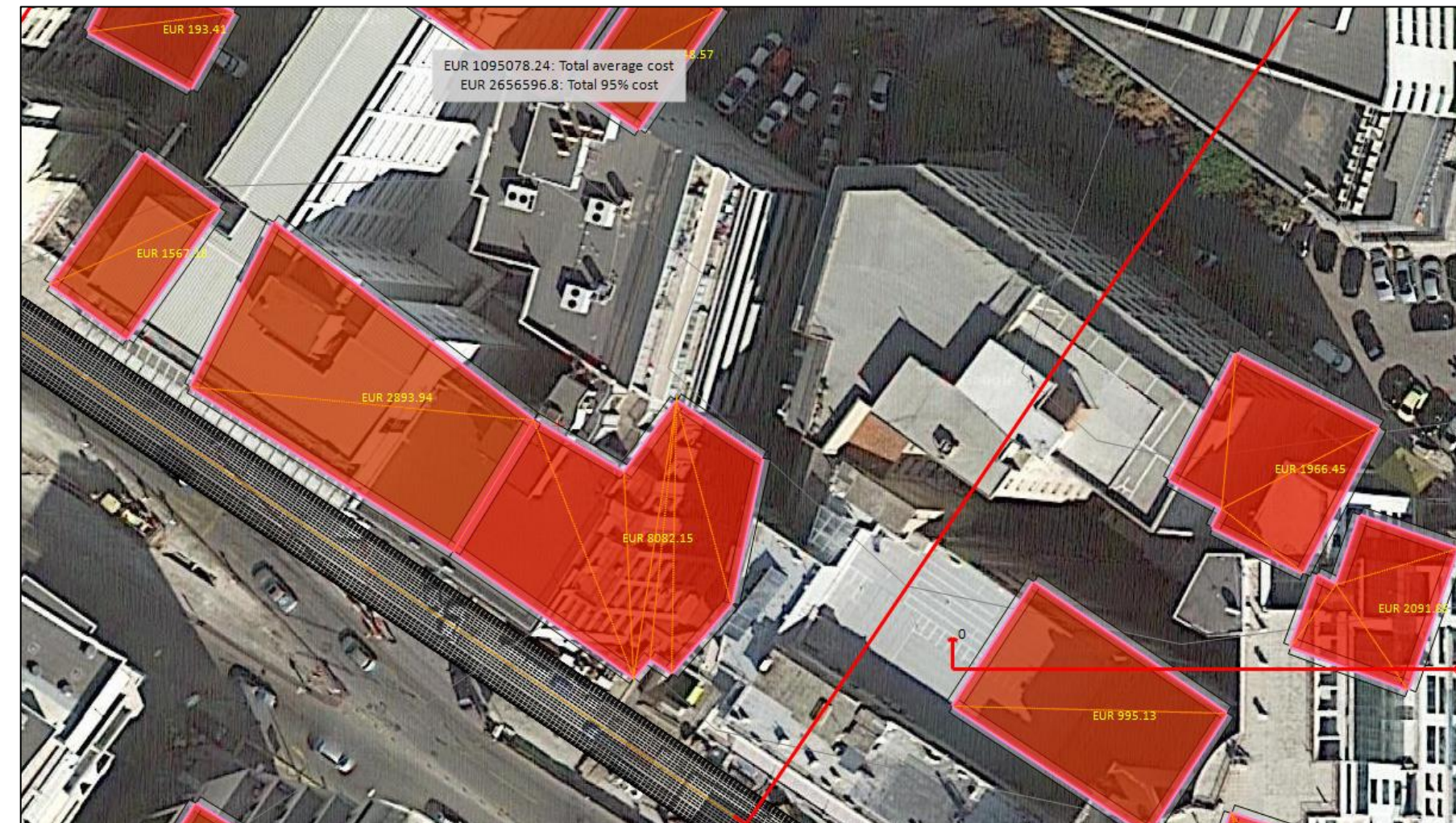
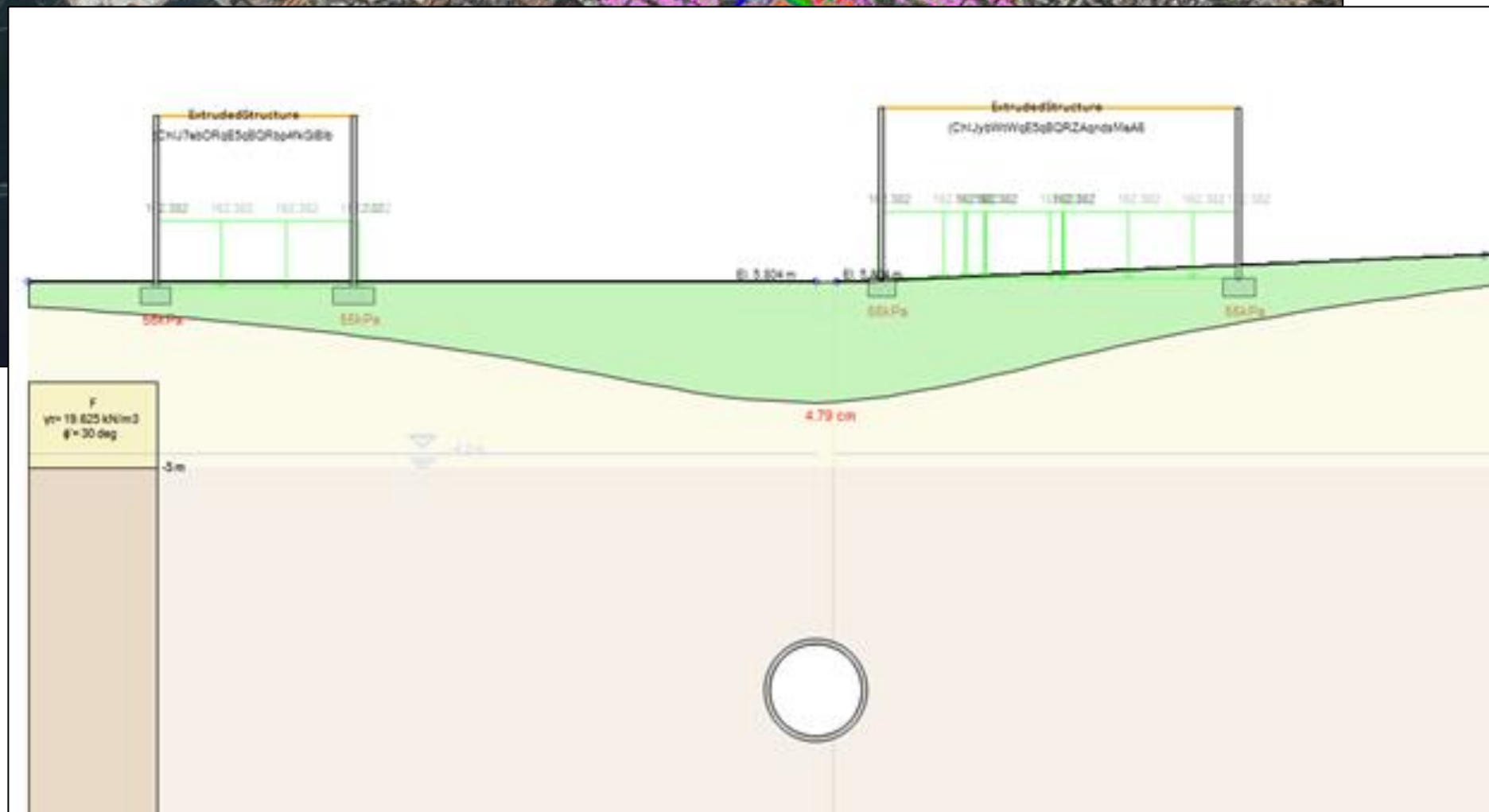
Viewing filters

Base model All borings All estimation sets All soil types



The Future is Here!

- ✓ Import your City Map with all Structures from Google
- ✓ Define your Tunnel Construction Stages and Location on the Map
- ✓ Automatically Generate 2D Cut Sections along your Tunnel
- ✓ Define your Metro Station Locations on the Map and Design Them
- ✓ Analyze the Tunnel, Calculate Settlements considering Soil Volume Loss, Consolidation and Water Drawdown
- ✓ Estimate the Damage Cost for all Imported Buildings
And more!





Projects Designed with DeepEX

Sweet Home Alabama! Uncommon Auburn

Courtesy of Russo Construction



Southbank Soil Nail Wall, Tempe AZ

PB&A + Deep Excavation



Secant Pile Wall For Maspeth Avenue Station, NY

Courtesy of Skanska



Javits Center Expansion, NY

EE Cruz + Deep Excavation

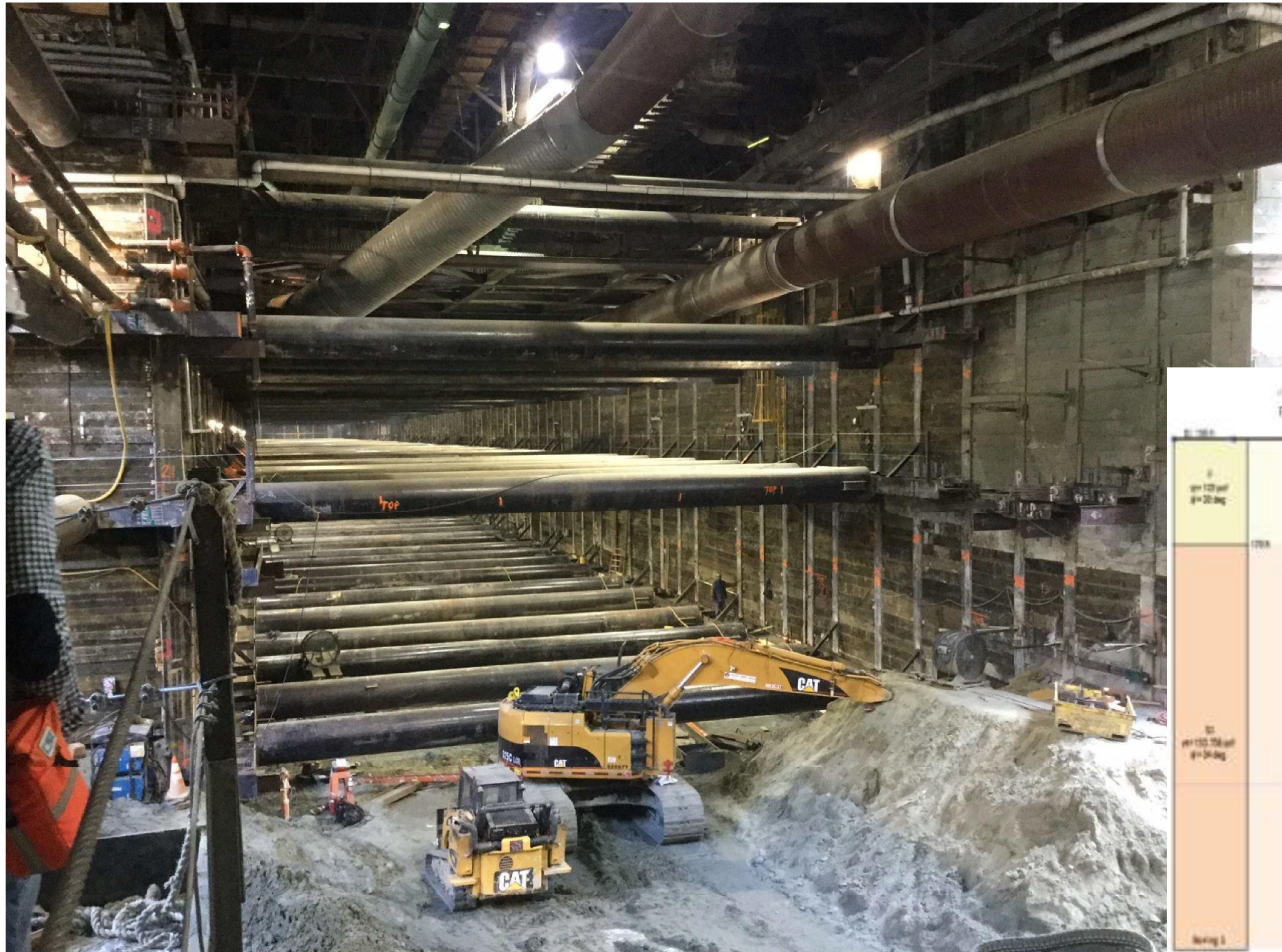


Over 3000 Users Professional Engineers & Firms
10000+ Projects Worldwide!

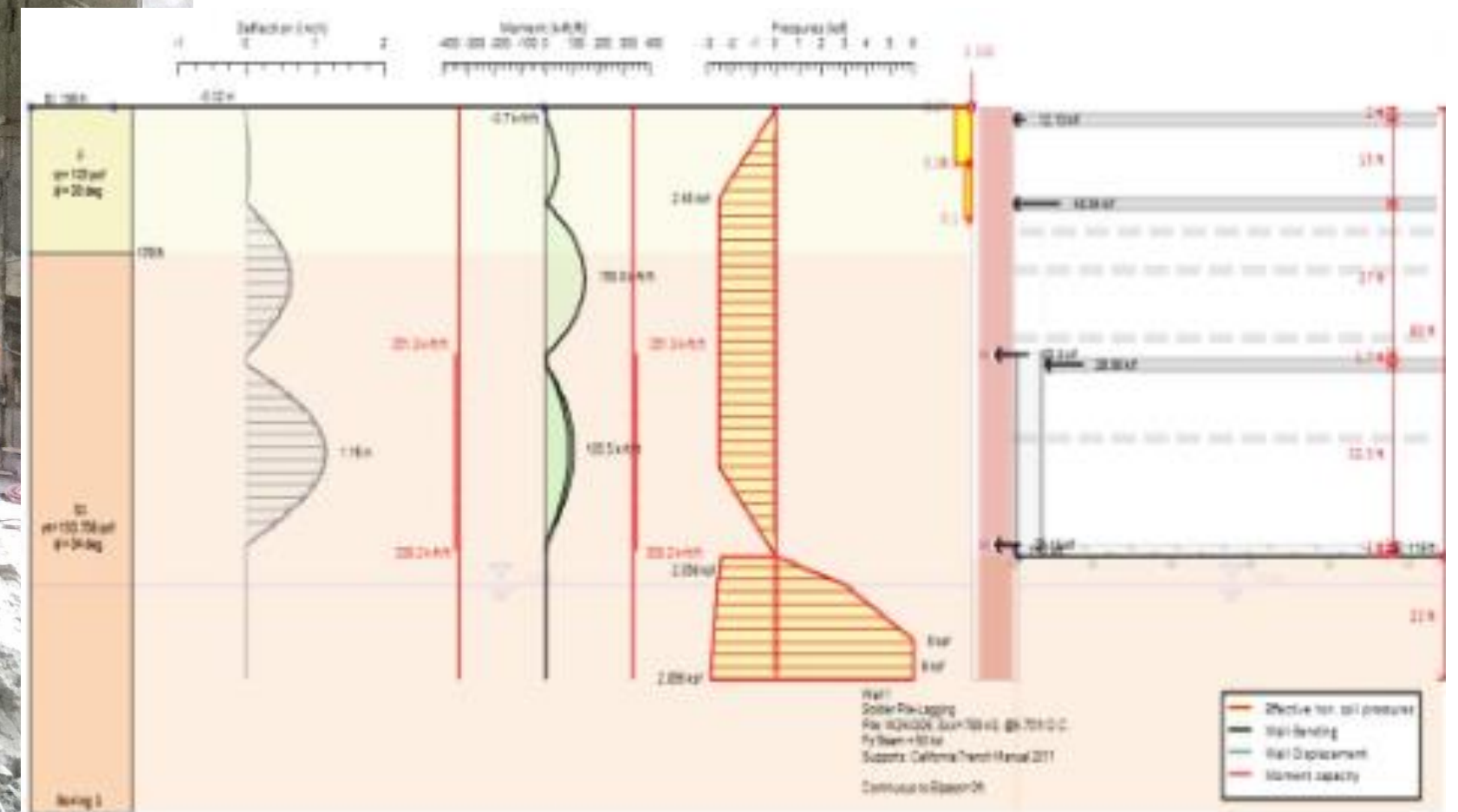
Access deepexcavation.com
Review Project Gallery



LaBrea Metro Station, Los Angeles, California, USA

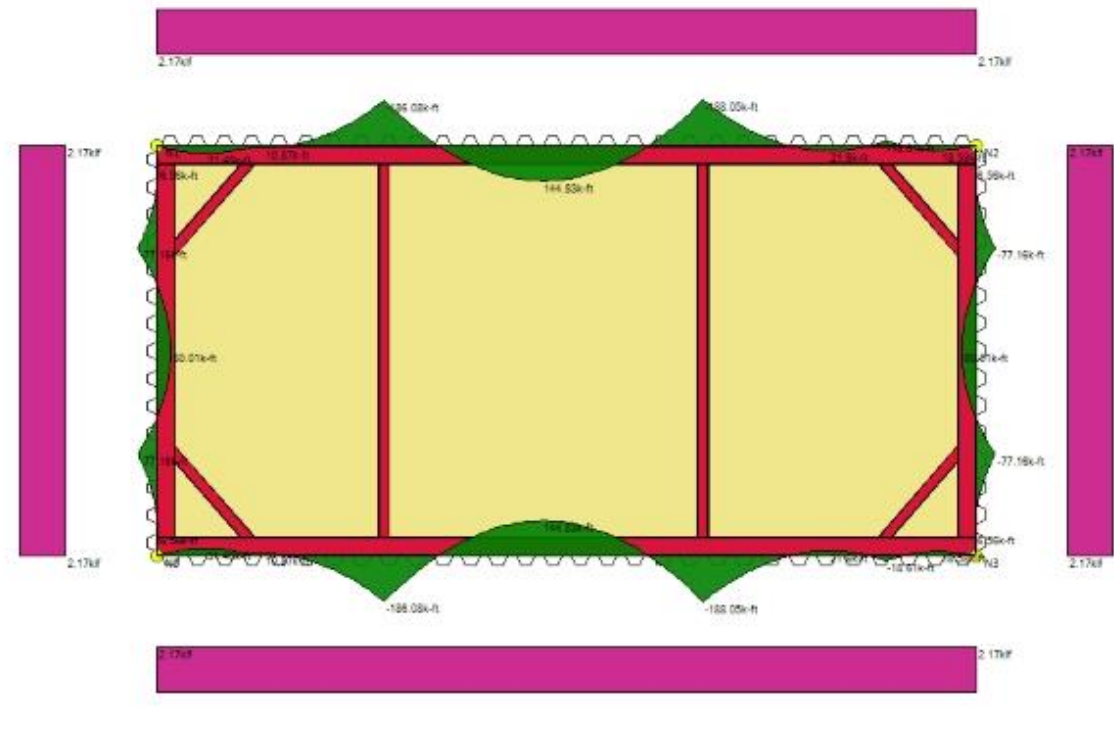
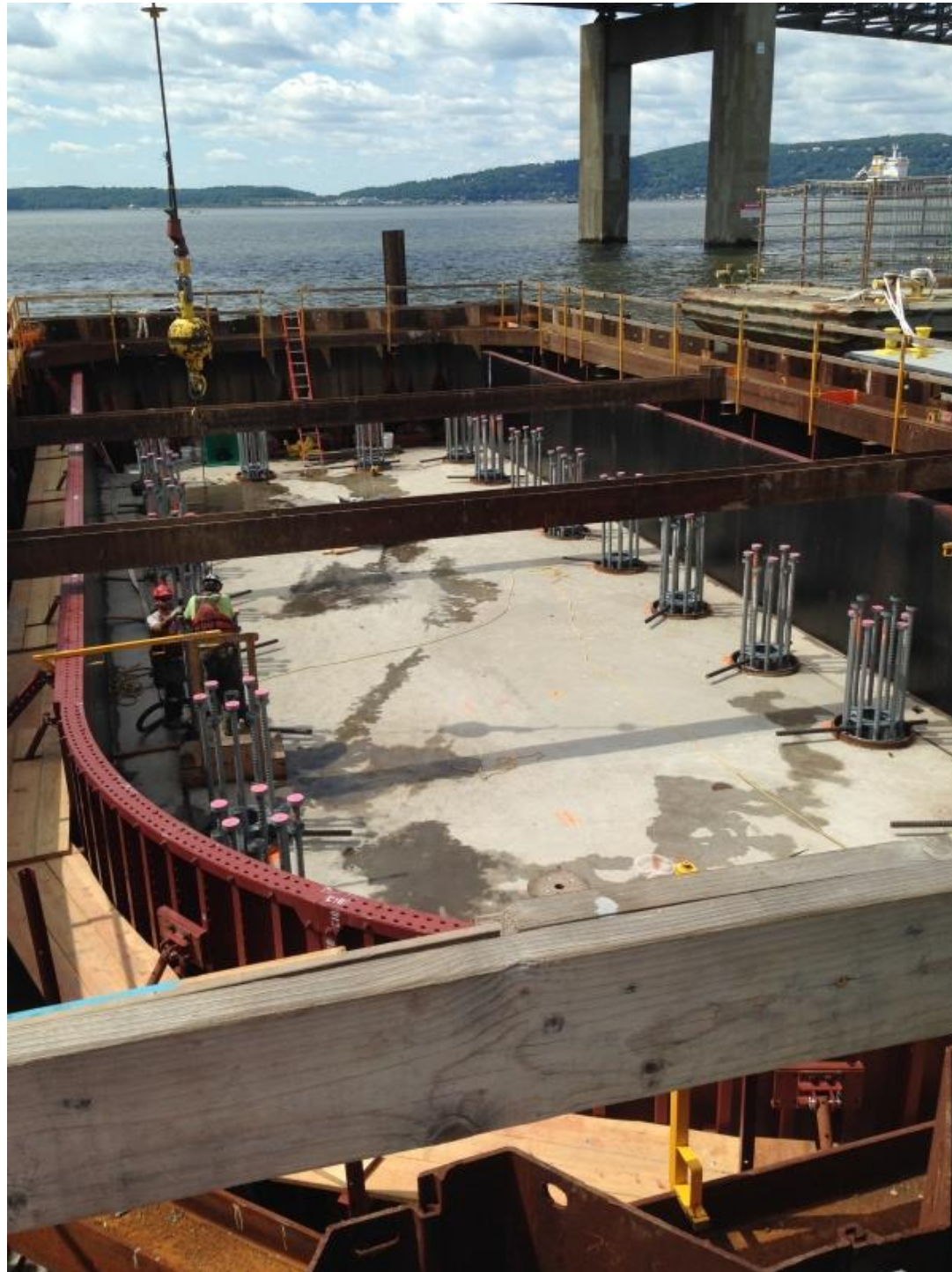


- ✓ 100 ft (30.5 m) Excavation
- ✓ Soldier Piles and Lagging
- ✓ Lateral Bracing (Struts)
- ✓ Full Design with DeepEX
- ✓ 5 Stations designed and under construction





New Tapan Zee Bridge Cofferdams, New York, USA



- ✓ \$3.9 billion project
- ✓ 90x45ft (27.5x13.7m) Cofferdams
- ✓ Lateral Bracing (Struts)
- ✓ Full Design with DeepEX





DeepEX
Shoring Design Software

Sample DeepEX Projects



Soldier Pile Excavation Pits with Diagonal Struts and Tiebacks, Arkansas, USA



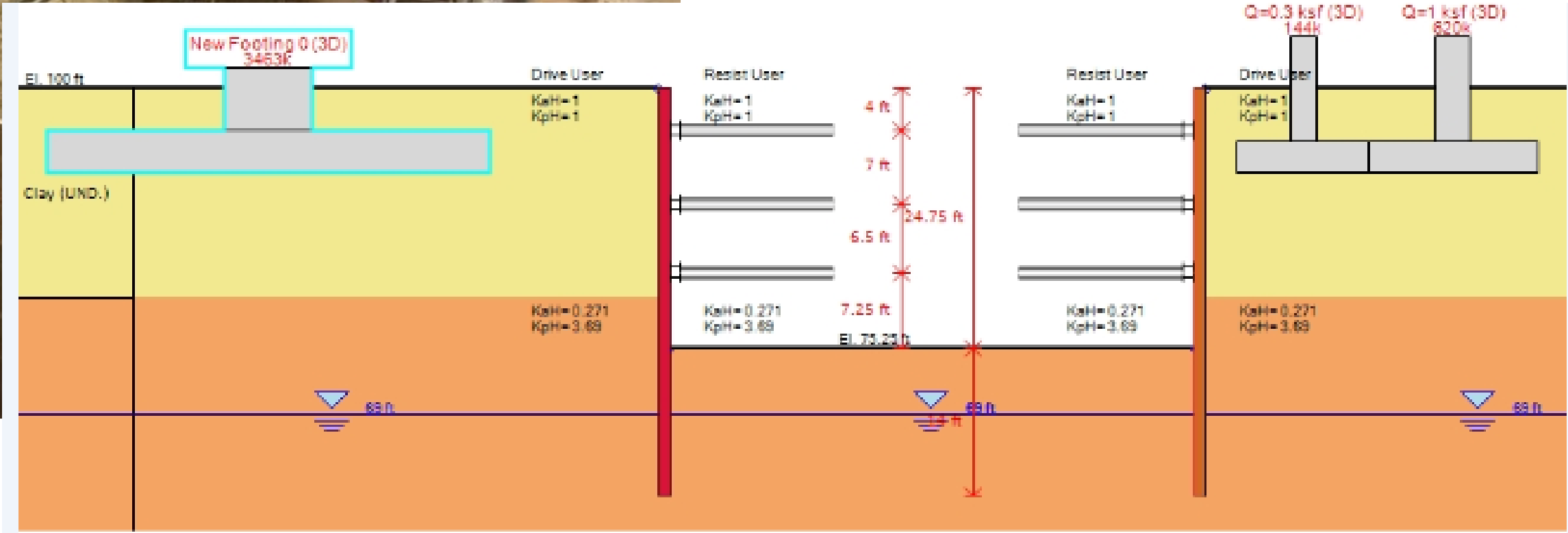
- ✓ 90 ft (27.5m) Excavation
- ✓ Soldier Piles and Lagging
- ✓ Pipe Struts
- ✓ 9 rows of Tiebacks



Soldier Pile Excavation Pits with Diagonal Struts, Arkansas, USA



- ✓ 24 ft (7.3m) Excavation
- ✓ Soldier Piles and Lagging
- ✓ Pipe Struts
- ✓ Full Design with DeepEX





DeepEX
Shoring Design Software

Sample DeepEX Projects



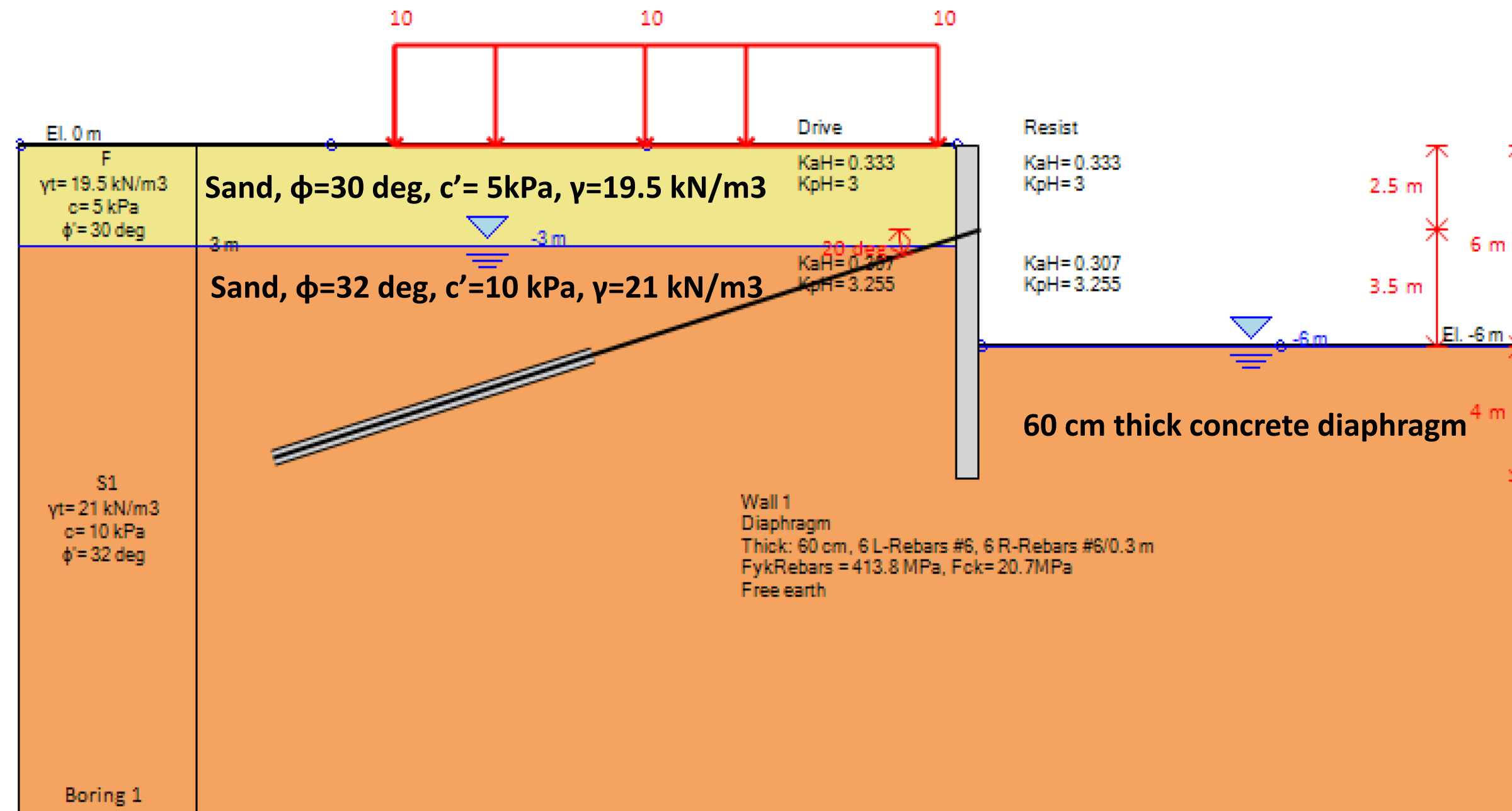
All American Canal, Imperial Irrigation District, Yuma, Arizona



- ✓ Cofferdam
- ✓ Water Wall Design
- ✓ Water Depth up to 20' (6m)
- ✓ Sheet Pile System
- ✓ Post Tension cable Ties
- ✓ Full Design with DeepEX



Example: A 6m Excavation - Anchored Diaphragm Wall



- ✓ Create model in DeepEX (all stages)
- ✓ Apply Eurocode 7 Specifications
- ✓ Run model with Limit Equilibrium Method
- ✓ Run model with Non-Linear Analysis Method
- ✓ Run model with Finite Element Analysis Method

- ✓ Review calculated results
- ✓ Optimize wall section
- ✓ Optimize support section
- ✓ Optimize anchor fixed length
- ✓ Optimize wall embedment



- Edit List of Soils
- Define Soil Properties for each soil

Top Elev.(m)	Soil Type	OCR	Ko	Edit
0	F	1	0.5	Edit
-3	S1	1	0.441	Edit

- Add Soil Layers
- Define Top Layer Elevation and Soil Type



Add Supports & Loads Graphically

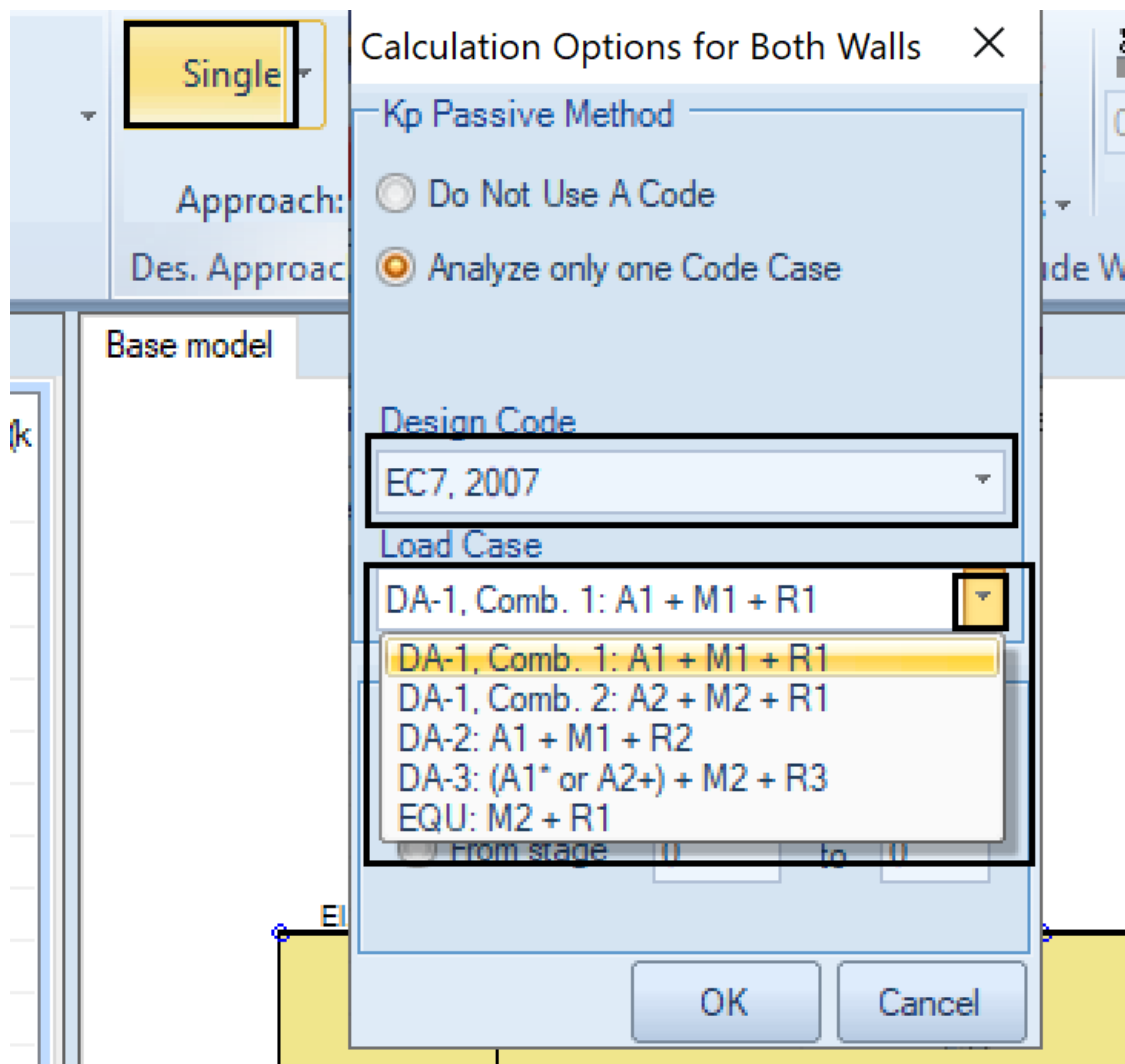
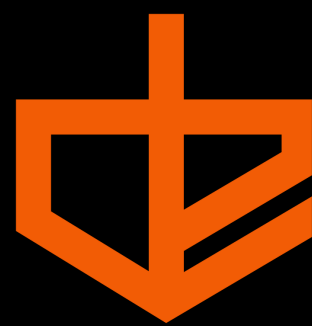


The screenshot shows the software interface with the 'Draw support' tool selected in the top toolbar. A dropdown menu lists various support types: Draw a ground anchor, Draw a strut support, Draw a slab support, Draw a raker support, Draw a fixed support, Draw a spring support, Draw a waler support, Draw a water beam on a support, and Draw a passive heelblock. The 'Edit Support Data' dialog box is open, showing fields for dimensions, angles, and prestress options. A '4-Strands' structural section is selected.

1. Select the Draw Support Tool
2. Click on the Wall
3. Click to the Ground
4. Define exact Support Location, Dimensions and Structural Section

The screenshot shows the software interface with the 'Draw loads' tool selected in the top toolbar. A dropdown menu lists various load types: Draw surface strip surcharge, Draw surface linear load, Draw a surcharge on the wall, Draw a line load on the wall, Draw Train-Embankment Load, Draw an external moment on a wall, Draw a prescribed displacement on a wall, and Edit Compaction Roller Loads. The 'Edit Distributed Load' dialog box is open, showing fields for coordinates, surcharge values, and type. The 'Is Surface Load' option is checked.

1. Select the Draw Loads Tool
2. Click on the Ground (Start Load Point - Left)
3. Click to the Ground (End Load Point - Right)
4. Define exact Load Location, Type and Magnitude



Eurocode 7 Approach

- Select a Load Combination
- Factors on the external loads and soil pressures
- Use a design surface load of 10 kPa
- Target for a wall embedment FS > 1

EC 7, DA-1, Comb.1

Code	EC7, 2007
Case	DA-1, Comb. 1: A1 + M1 + R1
Parameter	Safety Factor
Seismic multiplier	0
Variable loads	1.5
Permanent loads	1.35
Temporary anchors	1.1
Permanent anchors	1.1
tan(friction angle)	1
Eff. cohesion c'	1
Shear strength Su	1
Earth unfavorable	1.35 (STR)
Earth favorable	1
Water unfavorable	1.35
Water favorable	1
HYDraulic unfavorable	1.35
HYDraulic favorable	0.9
UPLift unfavorable	1.1
UPLift favorable	0.9
Used FS wall STR	1

EC 7 (GR), DA-2

Code	EC7-Greece
Case	DA-2*: A1 + M1 + R2
Parameter	Safety Factor
Seismic multiplier	0
Variable loads	1.5
Permanent loads	1.35
Temporary anchors	1.1
Permanent anchors	1.1
tan(friction angle)	1
Eff. cohesion c'	1
Shear strength Su	1
Earth unfavorable	1.35 (STR)
Earth favorable	1.4 (GEO)
Water unfavorable	1.35
Water favorable	1
HYDraulic unfavorable	1.35
HYDraulic favorable	0.9
UPLift unfavorable	1.1
UPLift favorable	0.9
Used FS wall STR	1

EC 7 (GR), EQU

Code	EC7-Greece
Case	EQU: M1 + R1 (GR)
Parameter	Safety Factor
Seismic multiplier	1
Variable loads	1
Permanent loads	1
Temporary anchors	1.1
Permanent anchors	1.1
tan(friction angle)	1
Eff. cohesion c'	1
Shear strength Su	1
Earth unfavorable	1
Earth favorable	1
Water unfavorable	1
Water favorable	1
HYDraulic unfavorable	1
HYDraulic favorable	0.9
UPLift unfavorable	1.1
UPLift favorable	0.9
Used FS wall STR	1
FS tiebacks	1x R_anchor



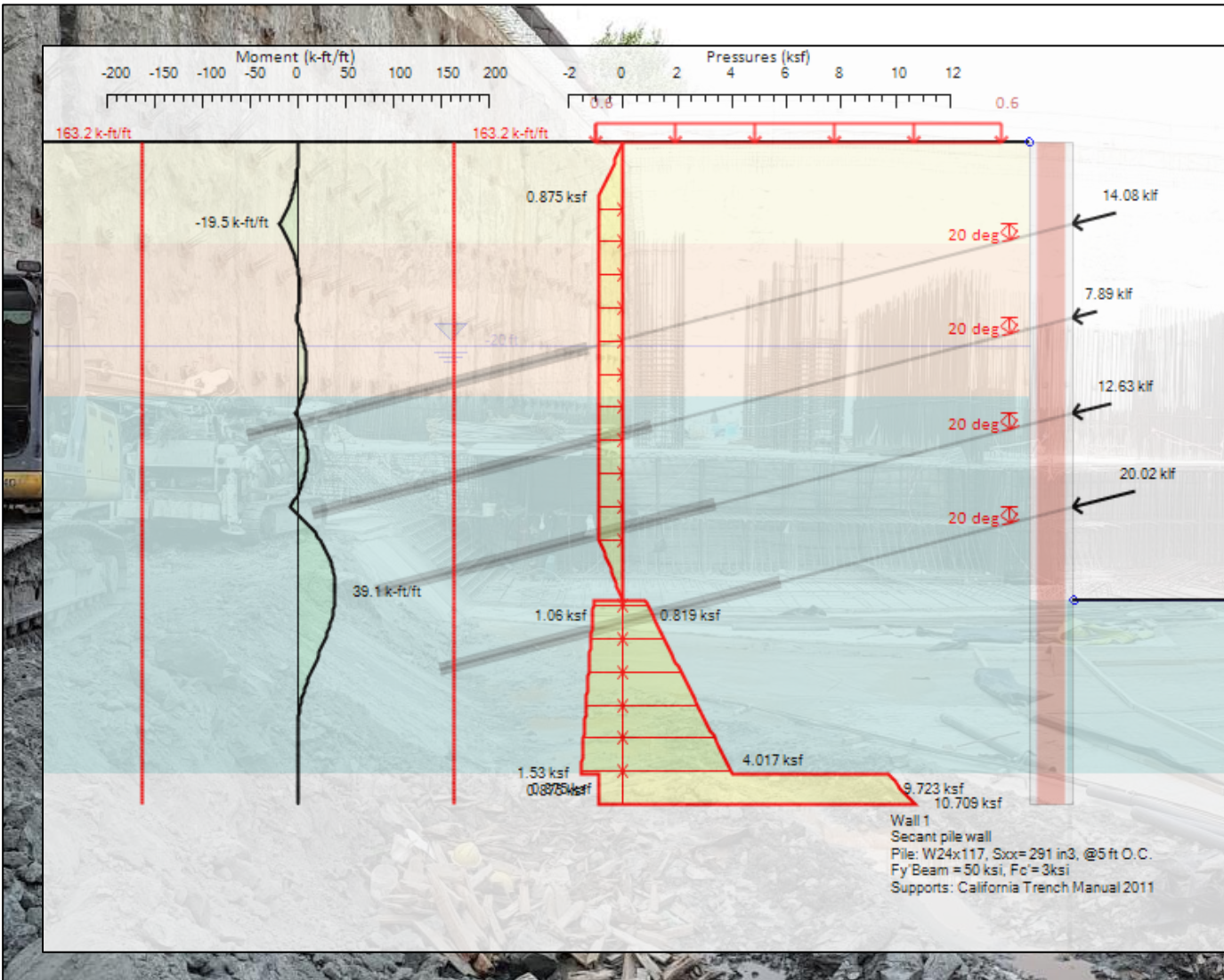
The screenshot shows the 'Analysis' tab in the software interface. The 'Type' is set to 'Limit equilibrium'. The 'Auto drive' option is checked. The 'Approach' is set to 'Service'. The 'Clays' are set to 'Default'. The 'Analysis' is set to 'Simple'. The 'Wave Forces' are set to 'Water behaviour'. The 'Thrust options' are set to 'Include Wall Friction'. The 'Drive Pressures' are set to 'Peck'. The 'Resist Pressures' are set to 'Passive'. The 'Supports' are set to 'Beam'. The 'Cantilever' is set to 'Free-earth'. The 'Beam' is set to 'Blum's method'. The 'Advanced Options' are set to 'Advanced'.

The screenshot shows the 'Design' tab in the software interface. The 'Code' is set to 'DE'. The 'General Settings' are set to 'Members: All (US, EUR)'. The 'Safety Factor' is set to '1'. The 'Divide Ultimate wall capacity by FS' option is checked. The 'Include axial load on walls' option is unchecked. The 'Steel corrosion' is set to 'Base model'. The 'Custom GEO FS' option is checked. The 'Safety Factor' is set to '1.35'. The 'Coef.STR' is set to '0.87'. The 'Use Soil Bond Values to calculate α otechn...' option is checked. The 'Tiebacks Safety' is set to 'User defined'. The 'Wall Embedment Optimization' option is unchecked. The 'Load Combinations' are set to 'LC'. The 'Use Load Comb.' option is unchecked. The 'Select Combination' dropdown is set to 'Base model'.

- Use Active pressures for the Cantilever excavation stage and Peck Apparent pressures for all stages with supports
- Examine Wall Friction (i.e. 33% of Soil Friction)
- Simplified Water Flow
- Cantilever Stage: Free Earth method
- Beam Analysis (stages with supports): Blum's Method
- Apply Eurocode 2,3 settings (Design Tab)



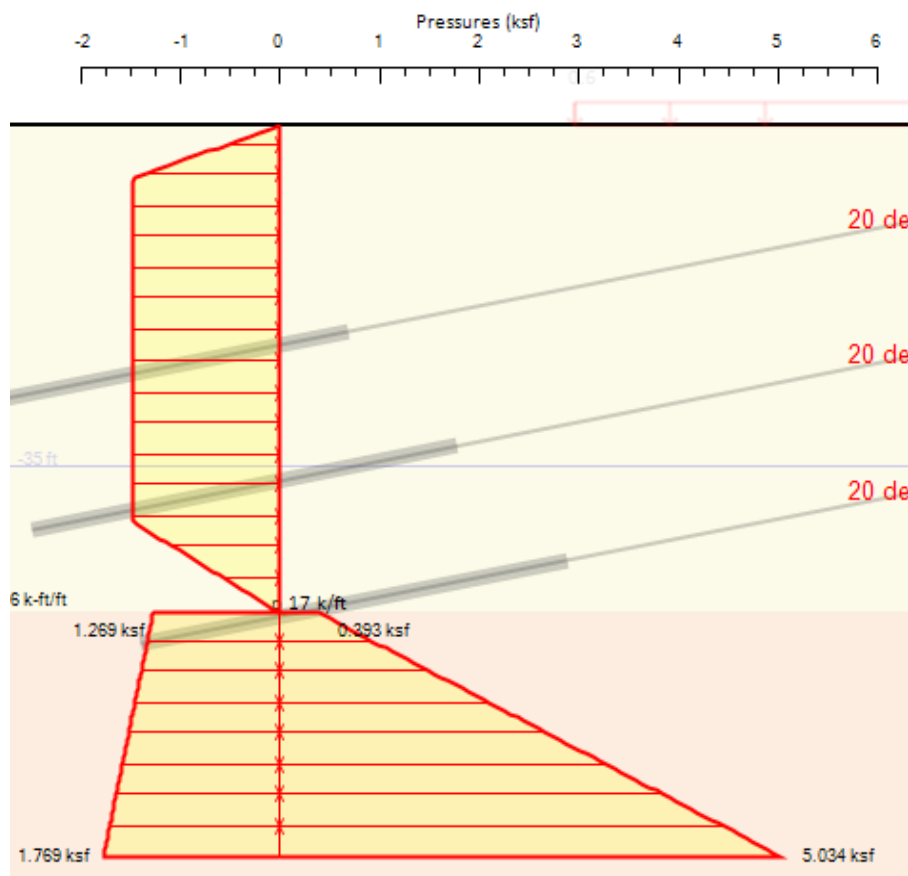
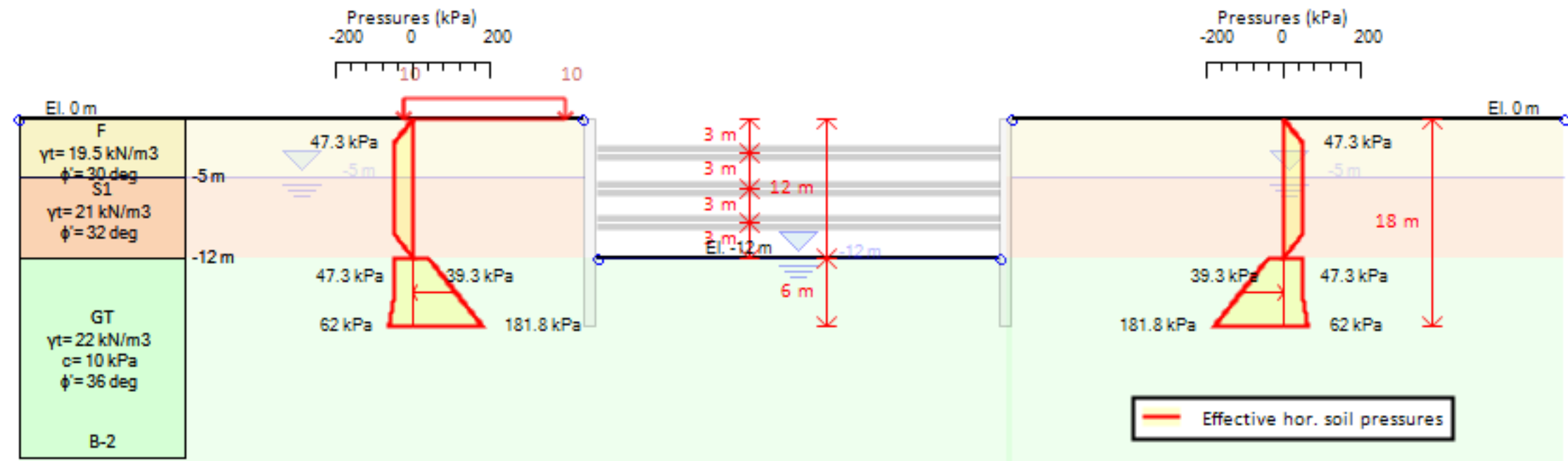
Theoretical Background & Methods



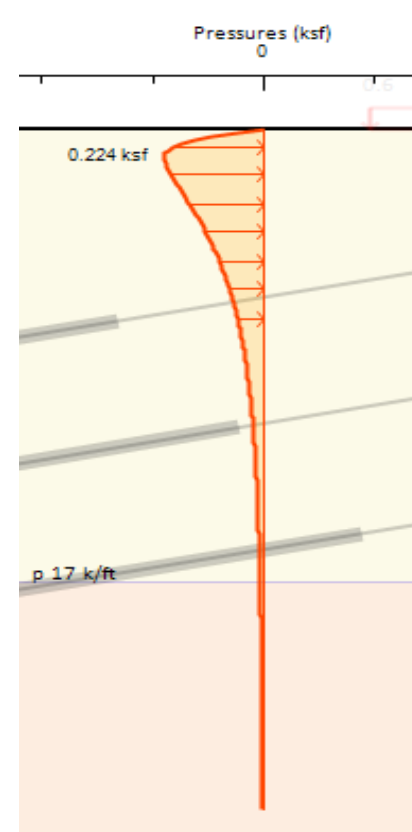
Access deepexcavation.com
Use of DeepEX - Wall Types & Support Systems



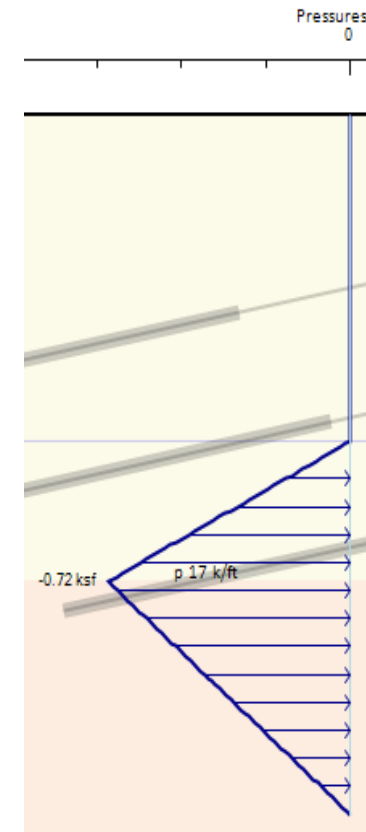
- ✓ Assume lateral earth pressures.
- ✓ Determine fixity locations for forces at subgrade.
- ✓ Analyze wall beam with assumed loads.
- ✓ Advantages: Easy method to verify. Gives a back check for more rigorous methods.
- ✓ Disadvantages: Soil-structure interaction ignored.



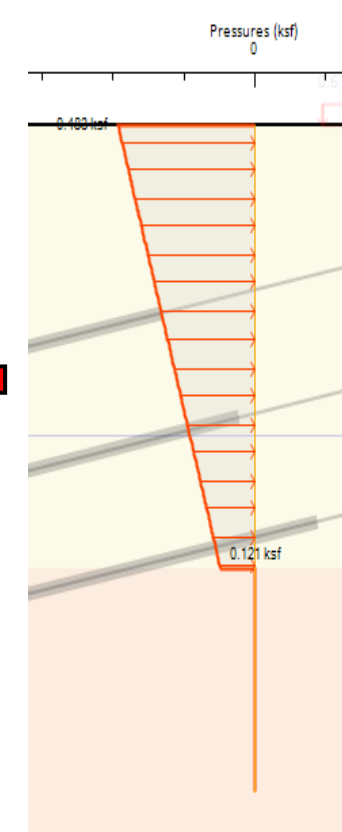
Soil Pressures



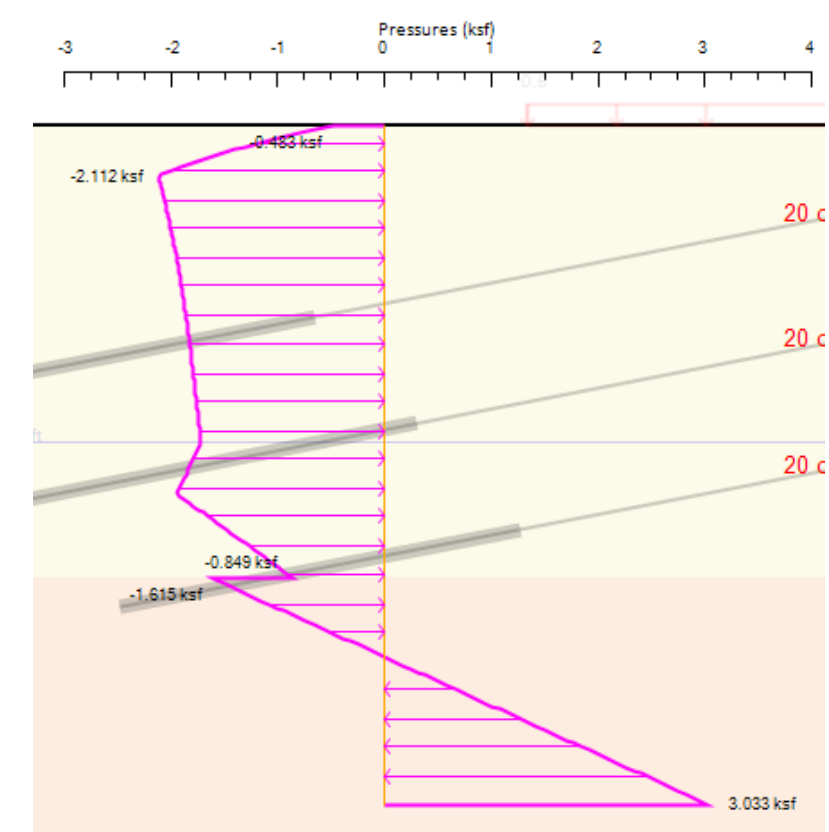
Surcharge



Water Pressures



Seismic Pressures

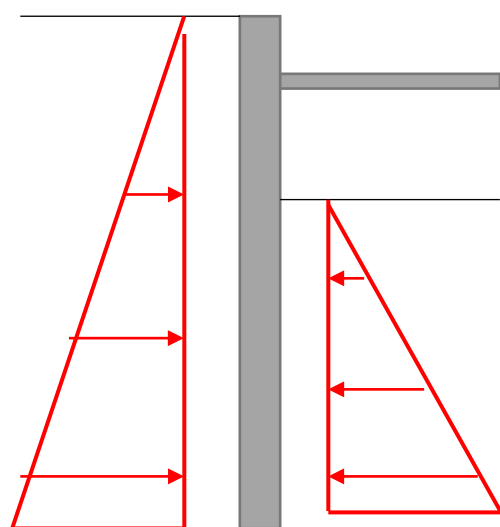


Net Pressures

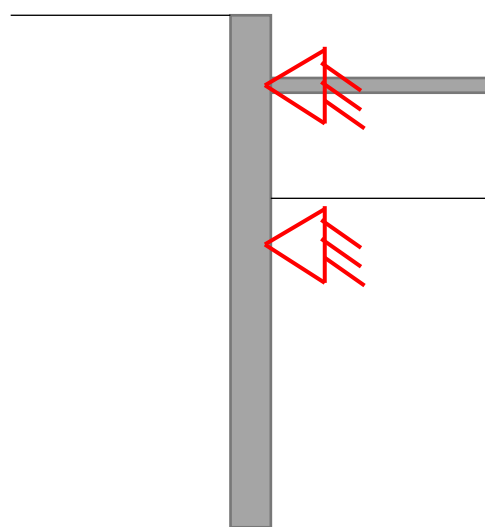


Limit Equilibrium Method (LEM) :

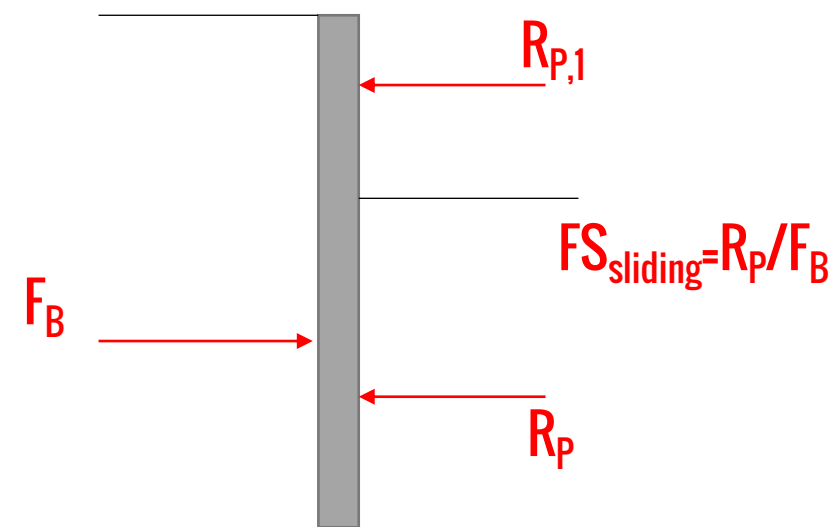
- Easy method to verify. Gives a back check for more rigorous methods.
- pre-failure strain is ignored
- Soil-structure interaction ignored



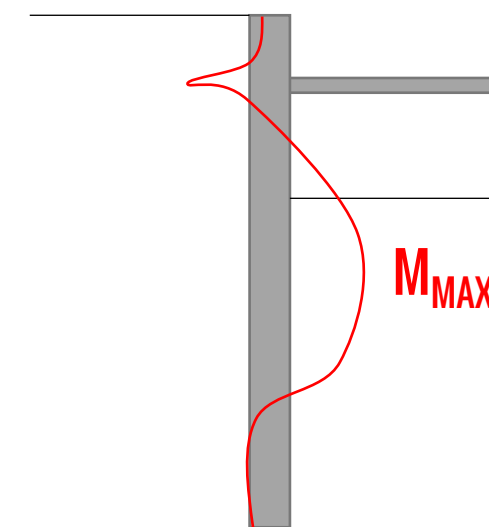
Calculation of lateral earth pressures.



Determine fixity locations and run beam analysis.



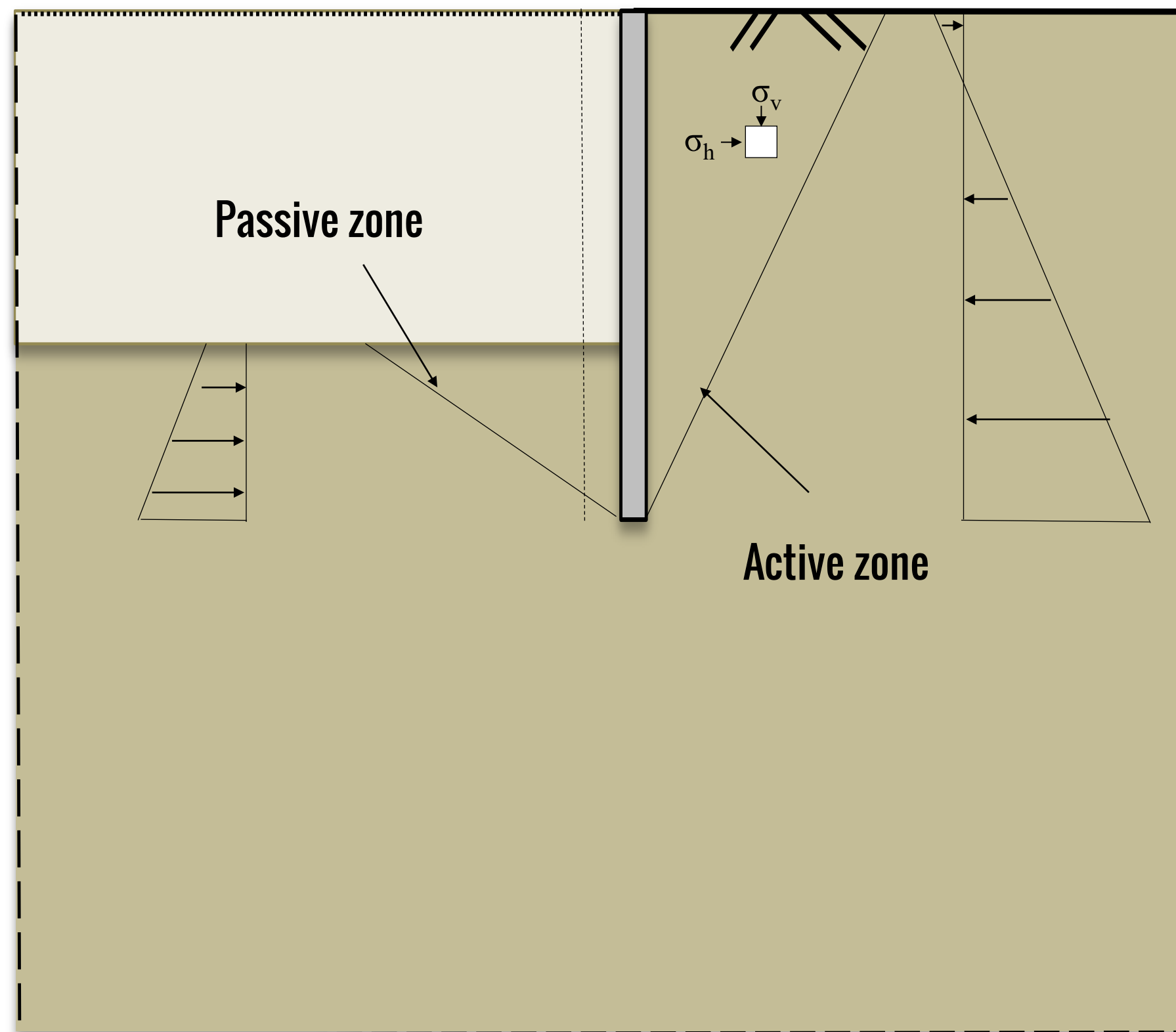
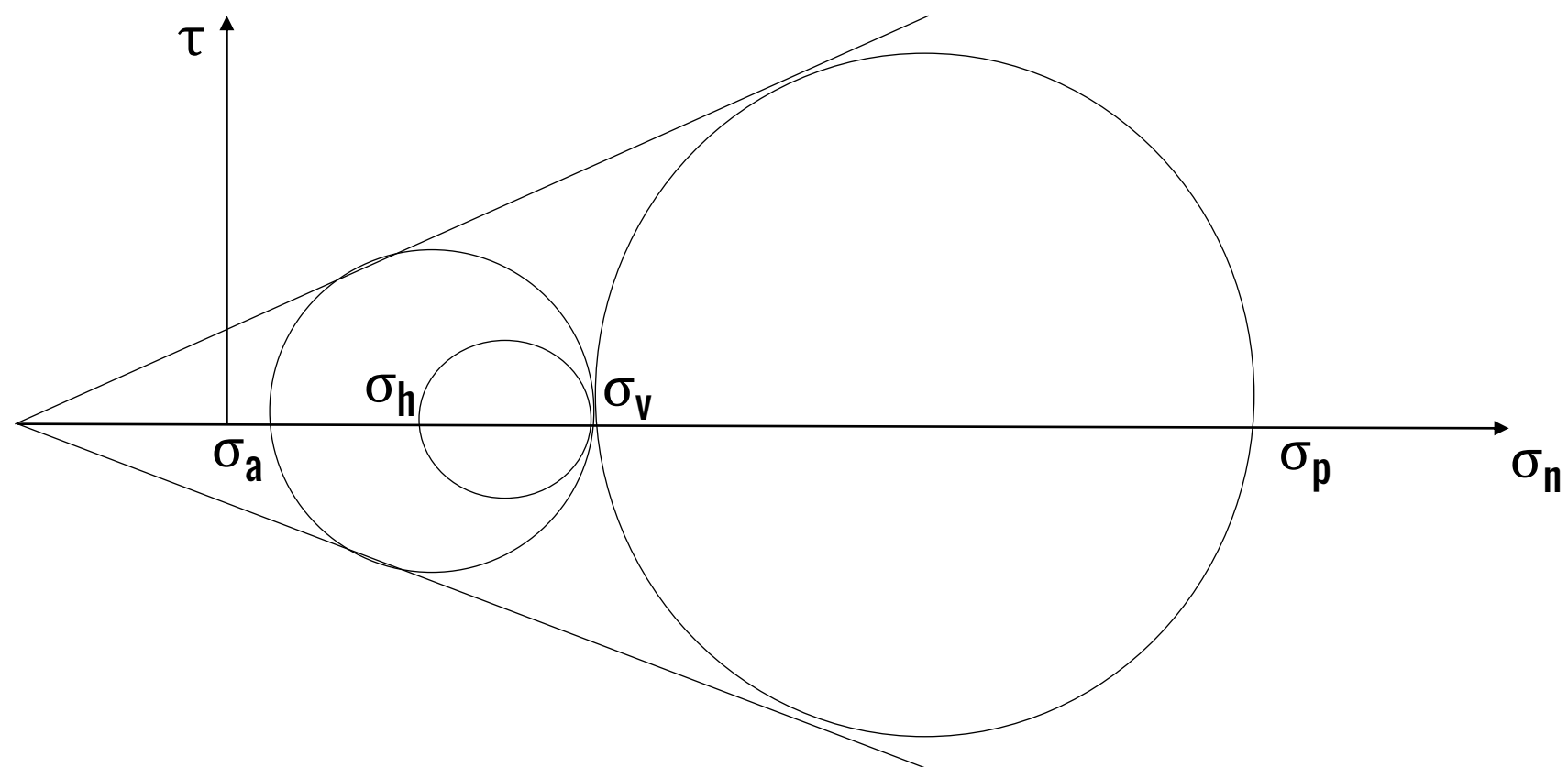
Calculate stability checks on the wall



Calculate structural design checks

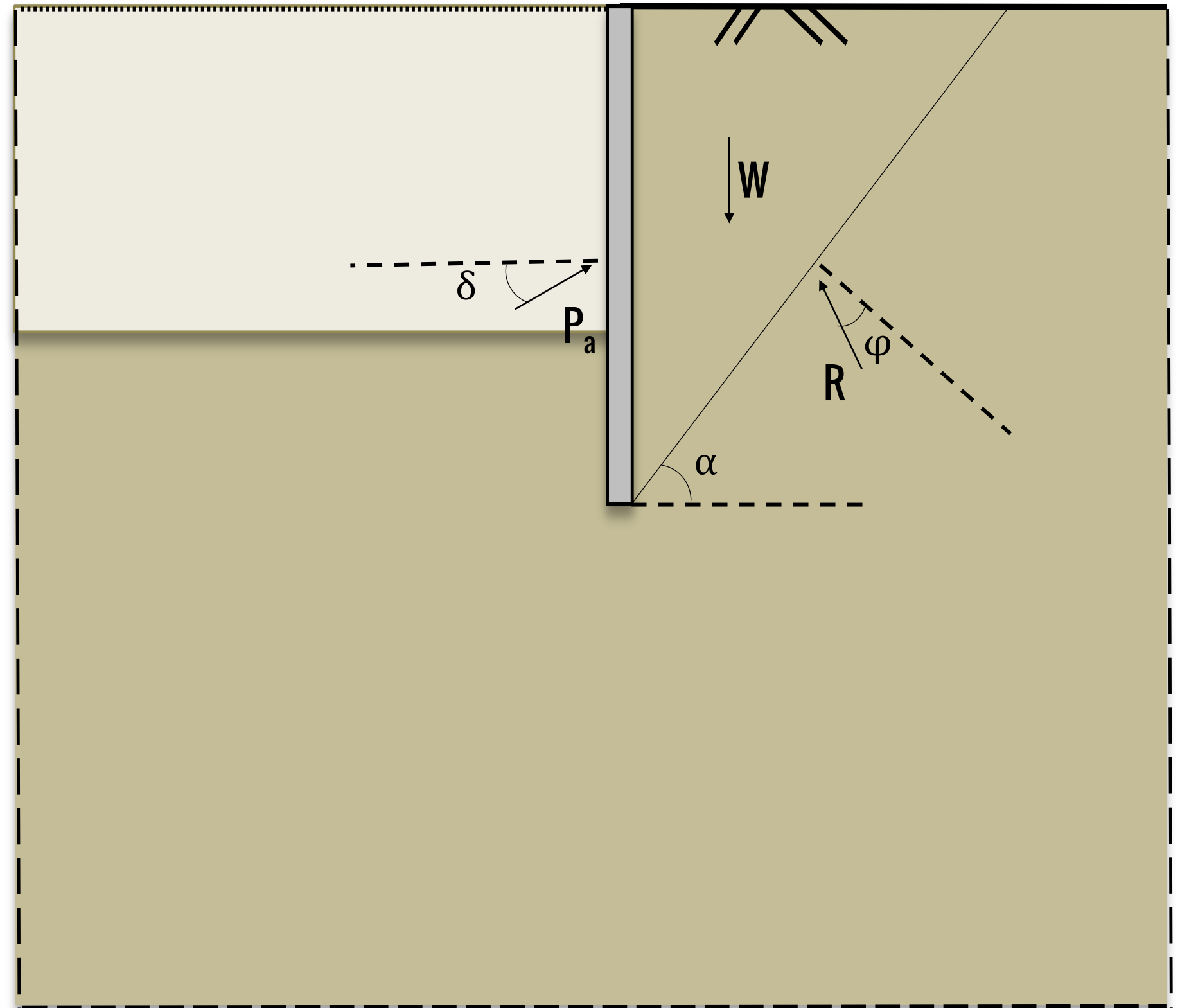
Rankine earth pressure theory (1857):

- Based on plastic equilibrium concept
- No friction between wall and soil



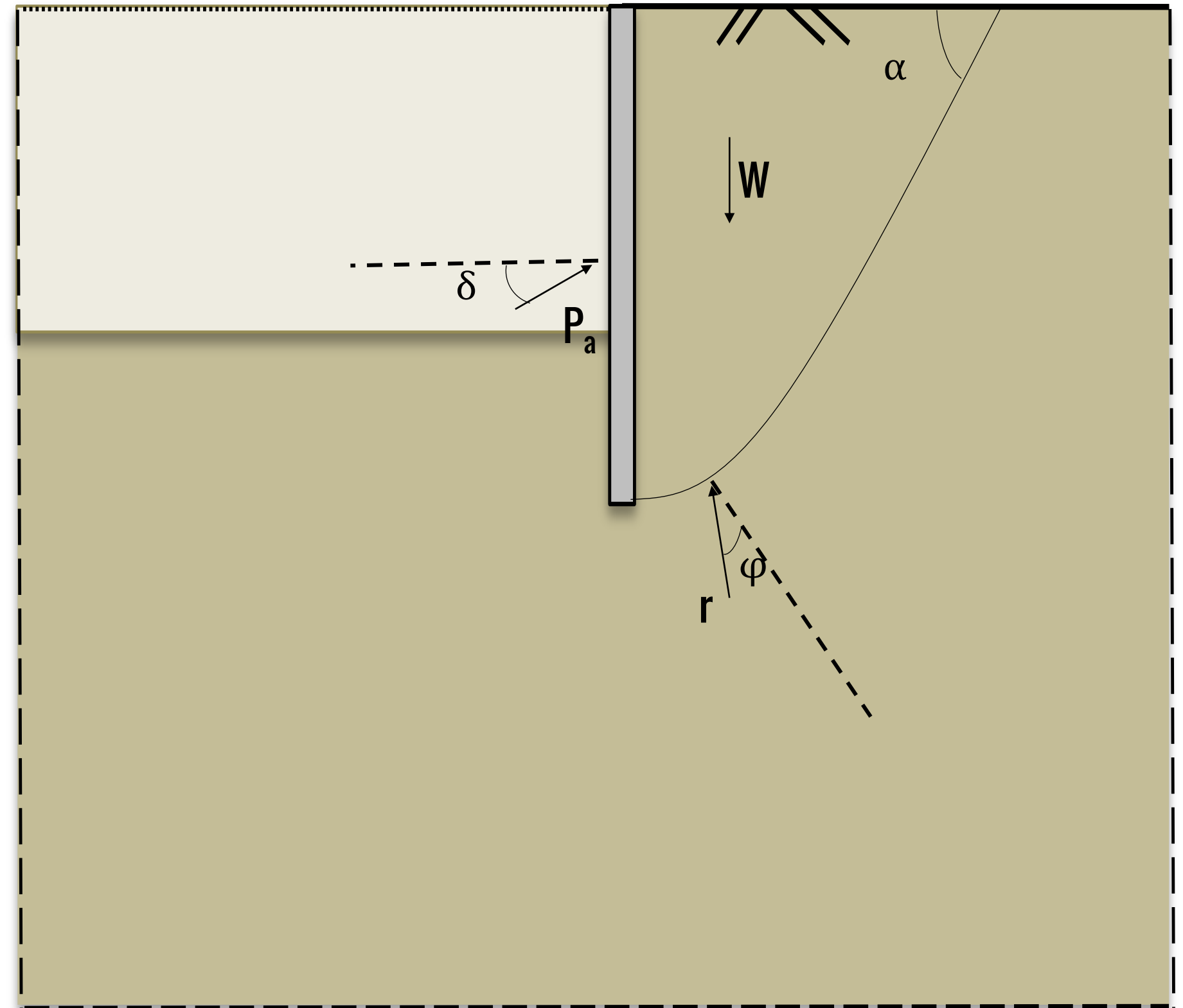
Coulomb earth pressure theory (1776):

- Failure surface is a plane
- friction between wall and soil can be considered
- Arbitrary slope at the sides of the wall



Caquot and Kerisel earth pressure theory (1948):

- Failure surface is an elliptical curved plane
- Eurocode 7 and NAVFAC charts based on the theory

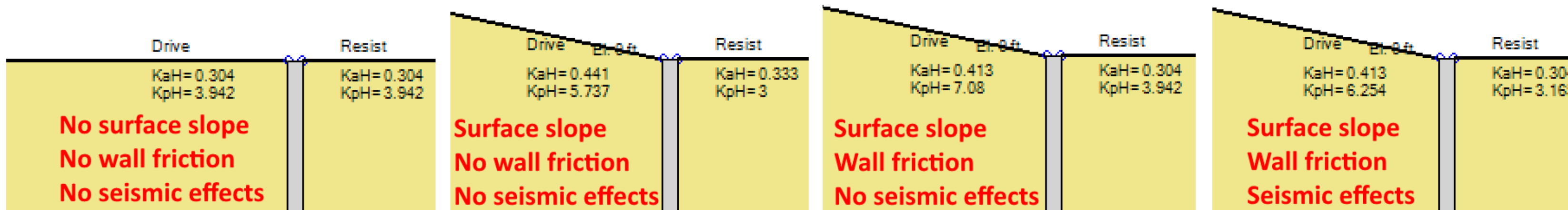




DeepEX Automatic Method Selection According to Project Parameters

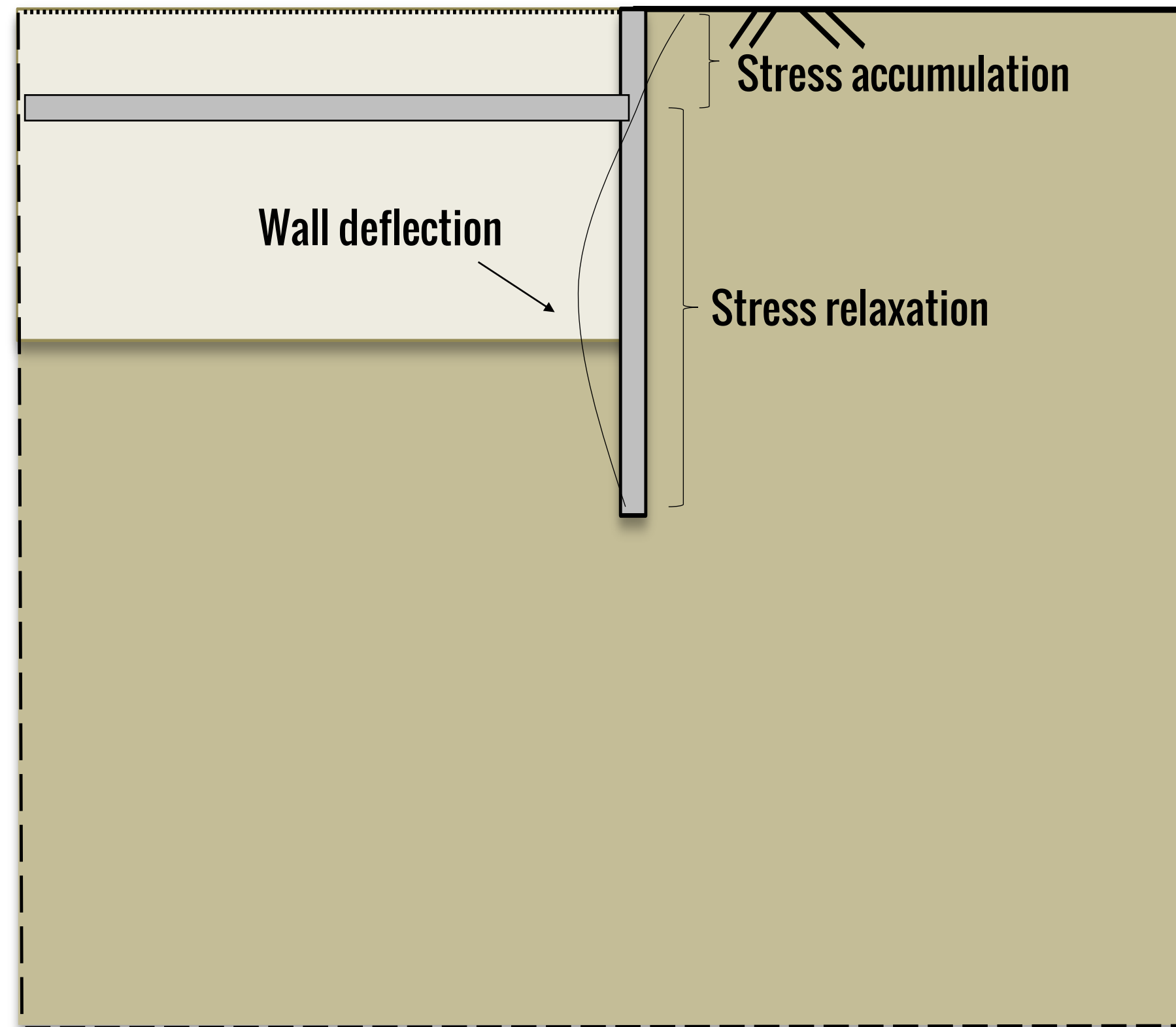
Active Coefficient K_a				
Parameters	Horizontal Surface	Inclined Surface	Wall Friction Considered	Seismic Effects Applied
Method	Rankine	Coulomb	Coulomb	No Effect

Passive Coefficient K_p				
Parameters	Horizontal Surface	Inclined Surface	Wall Friction Considered	Seismic Effects Applied
Method	Rankine	Coulomb	Caquot-Kerisel	Lancelotta



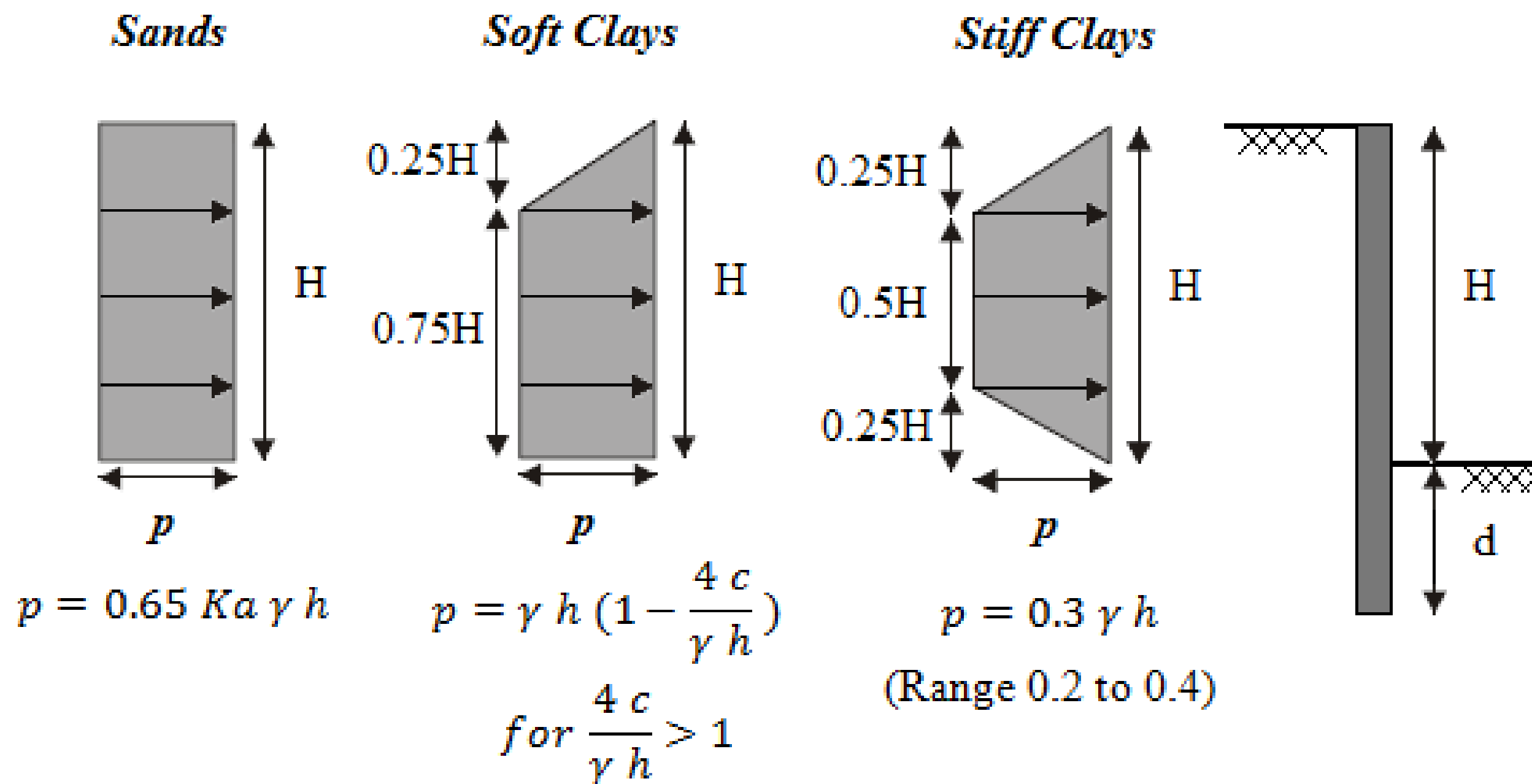
Apparent earth pressures

- Multilevel supported excavations experience different earth pressures
- Effective active pressures multiplied by a factor and redistributed.



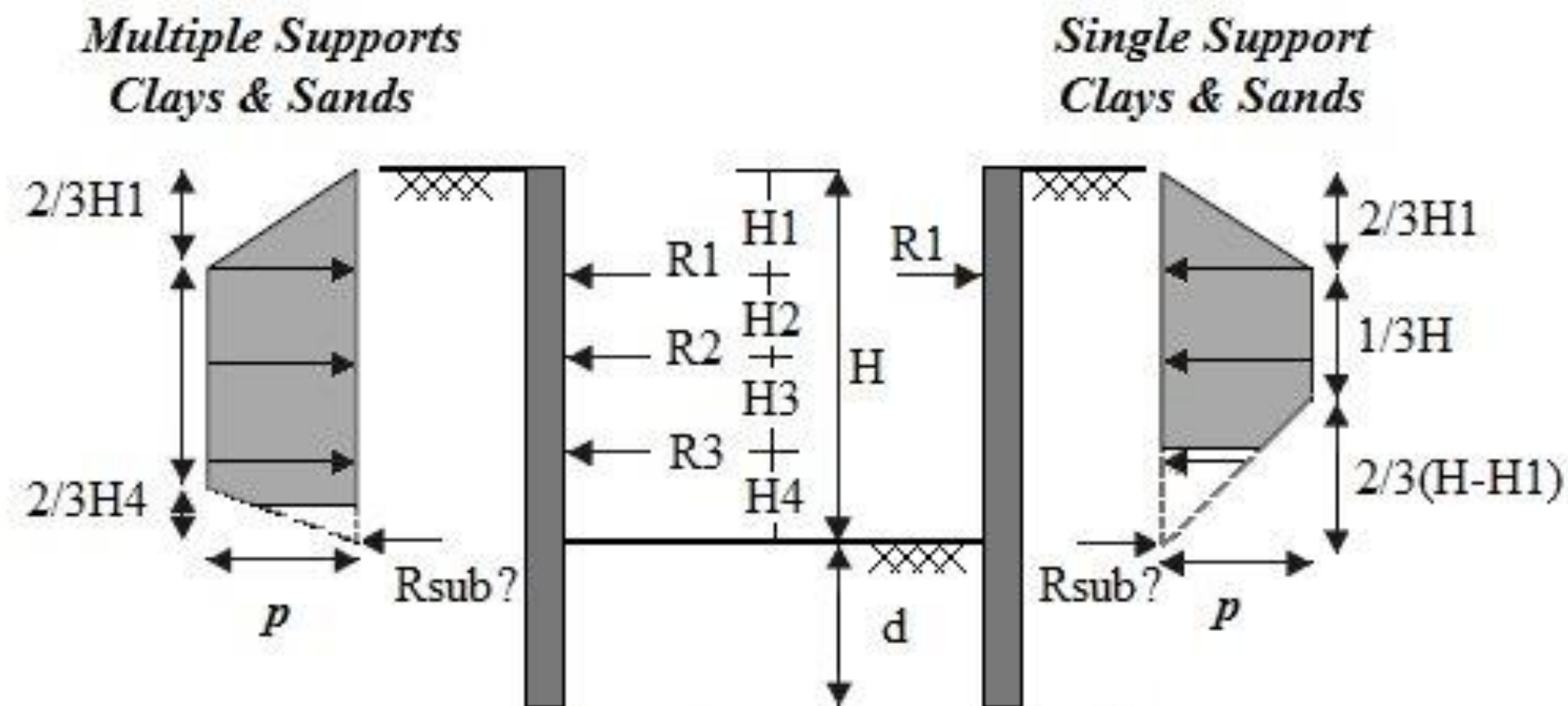
Peck (1969) Method:

- Based on multiple measurements of strut reactions of strutted excavations



FHWA Method:

- Based on multiple measurements of strut reactions of strutted excavations



Sands: Total Load = $0.65 K_a \gamma h^2$

Clays: $p = 0.2 \gamma h$ to $0.4 \gamma h$

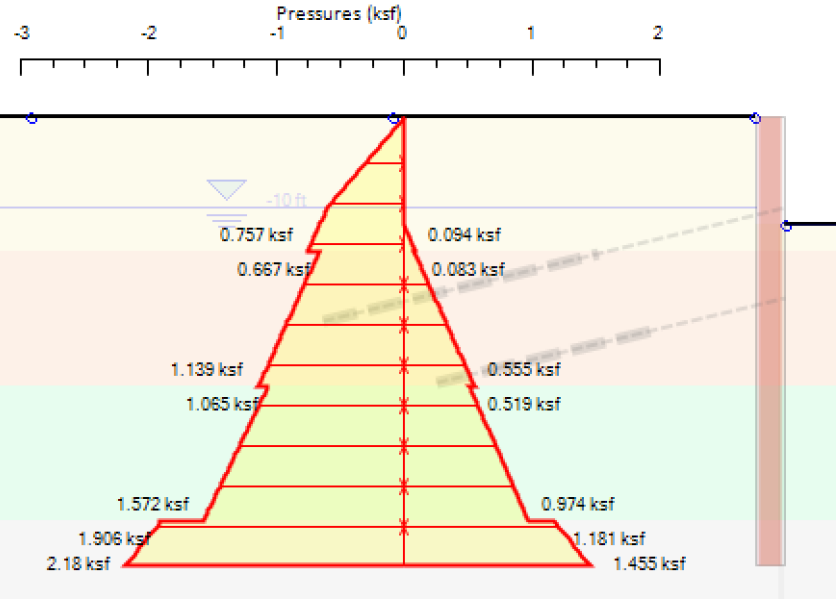
R_{sub} = Apparent virtual reaction at subgrade



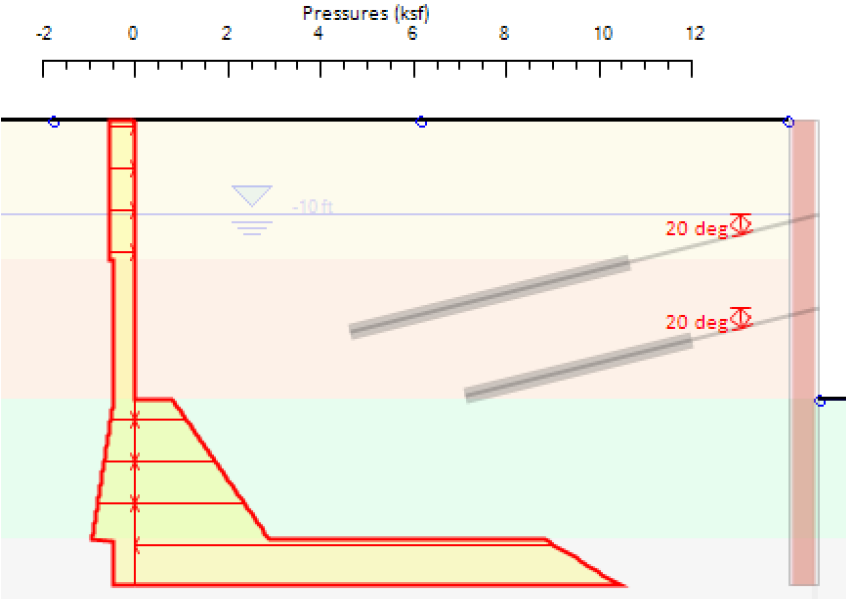
Cantilever Excavations

Construction Stages with multiple support levels

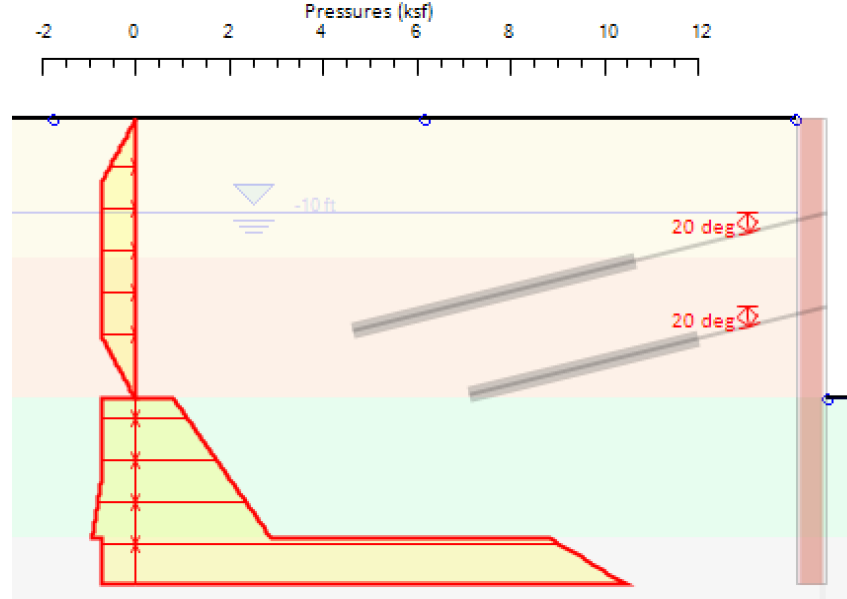
At-Rest Pressures



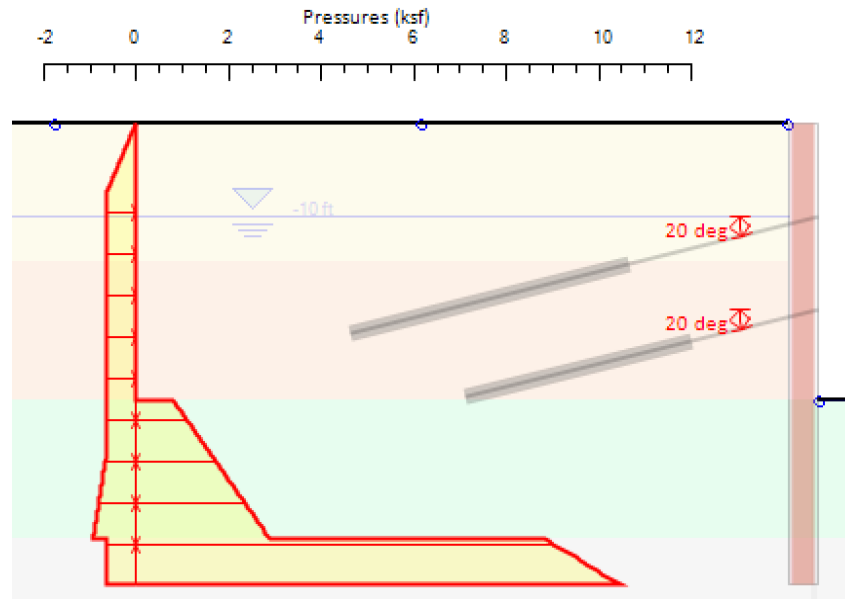
Peck 1969 Pressures



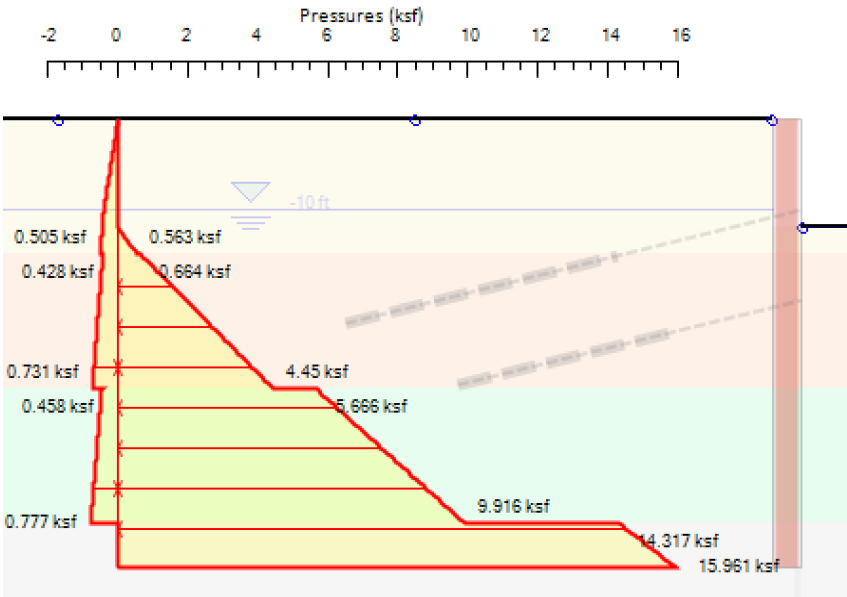
FHWA Apparent Pressures



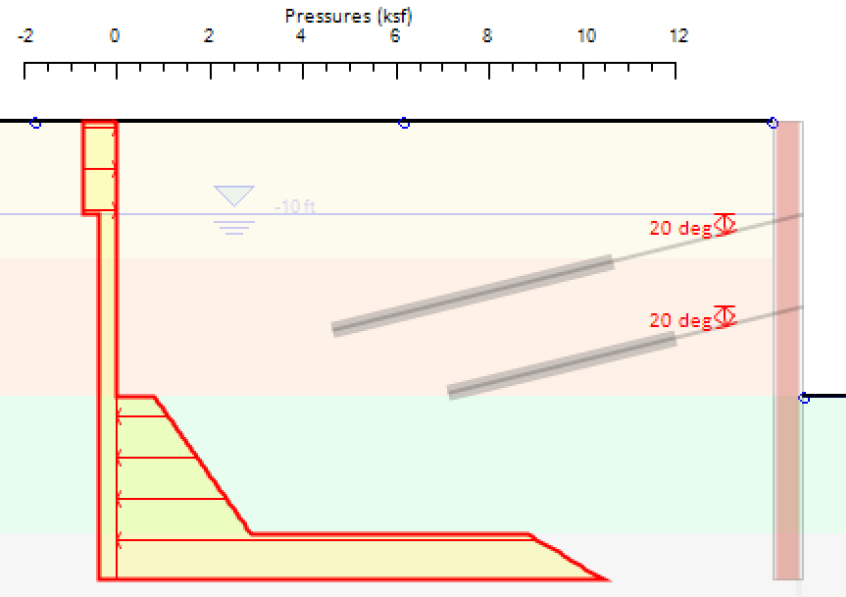
Custom Trapezoidal Pressures



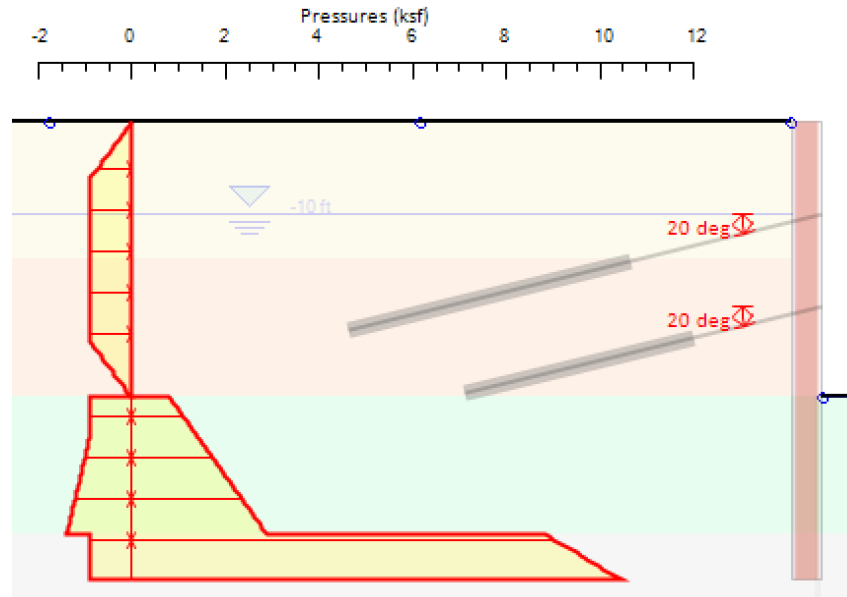
Active - Passive Pressures



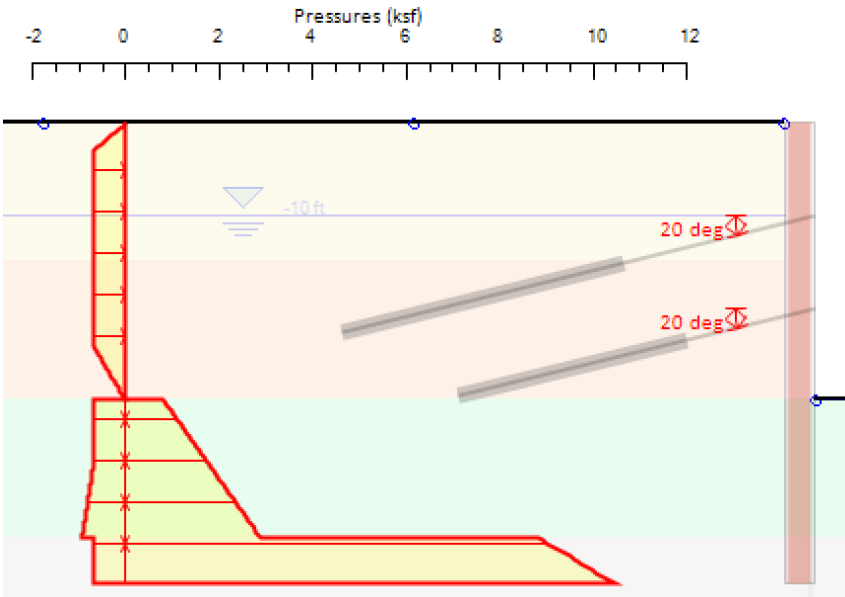
Two-Step Rectangular Pressures



WMATA Pressures



New York City DEP Pressures

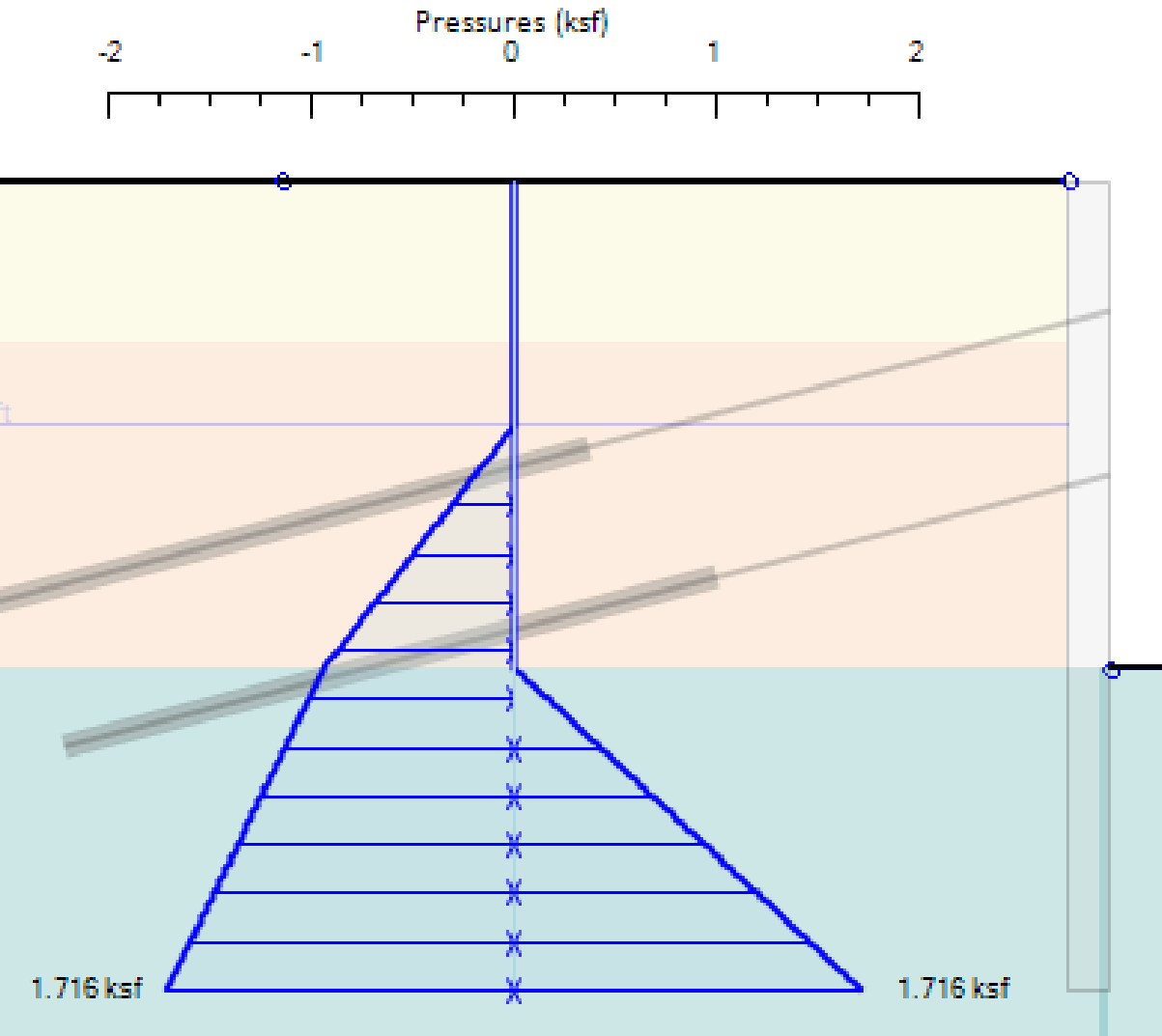




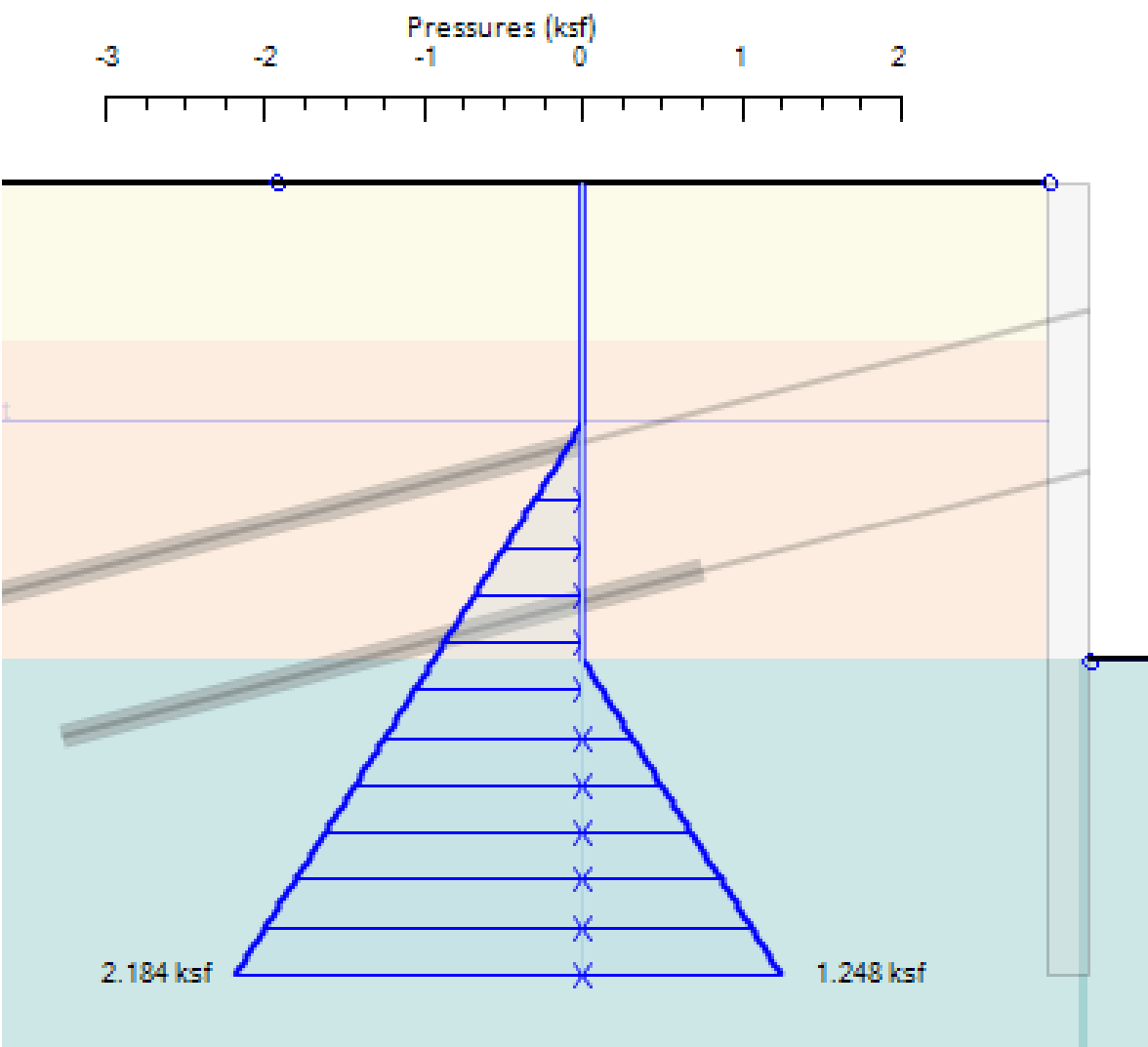
Water Pressure Methods



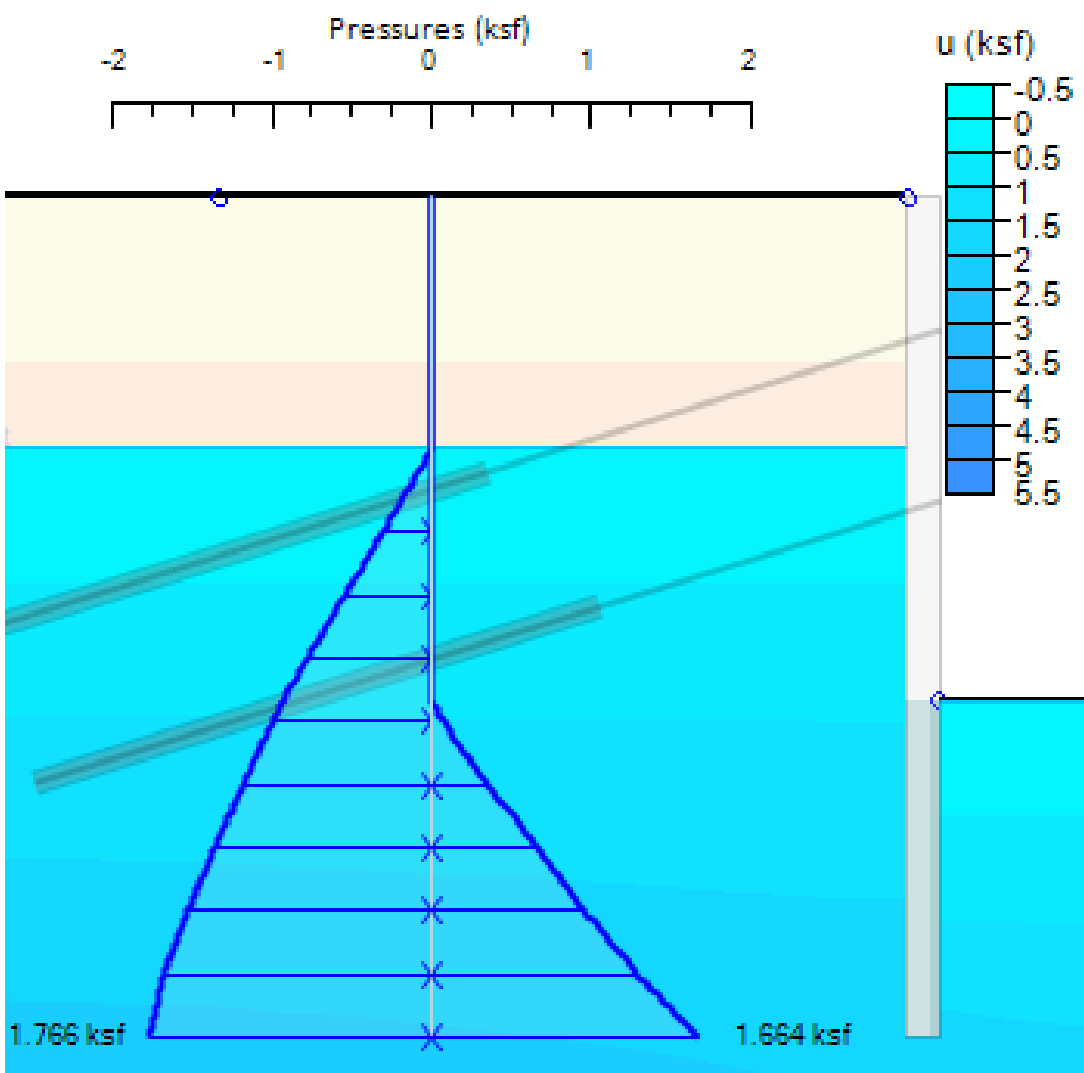
Simplified Flow

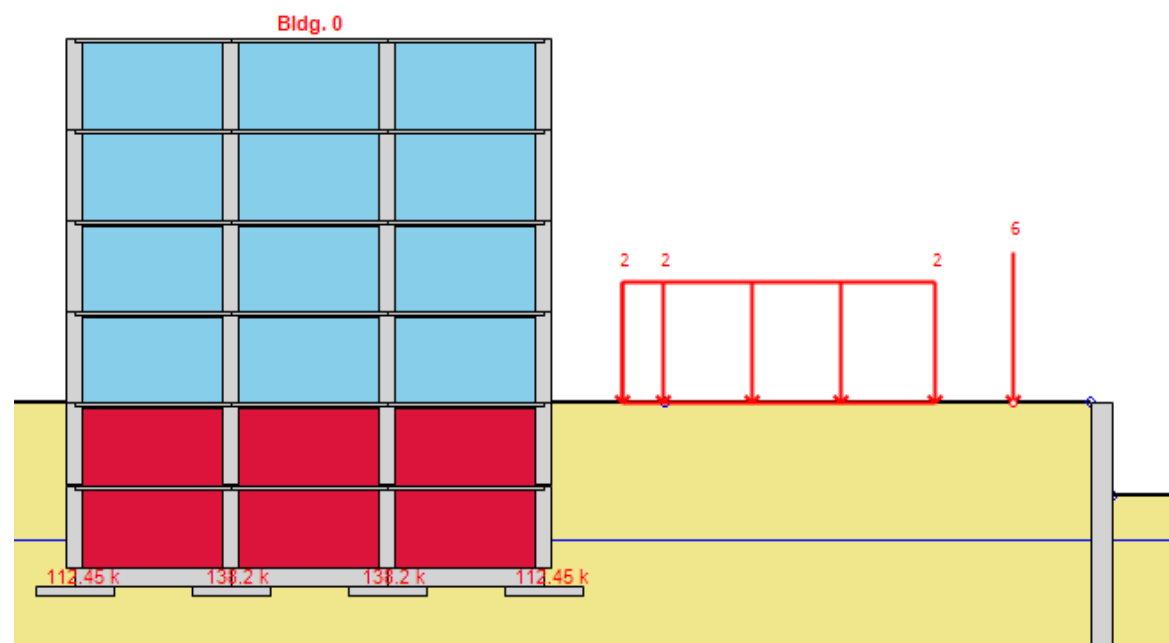


Hydrostatic



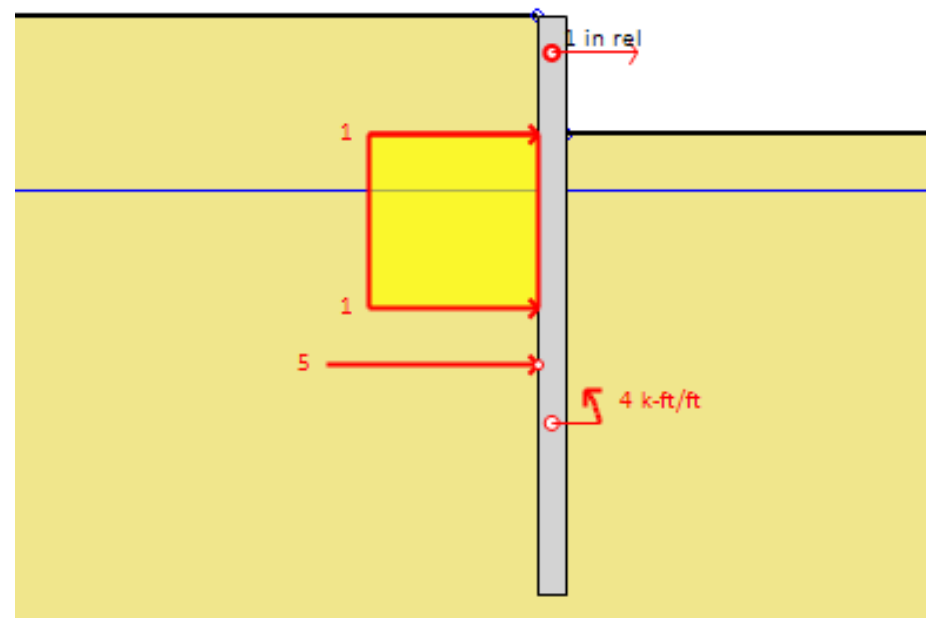
Full Flownet Analysis





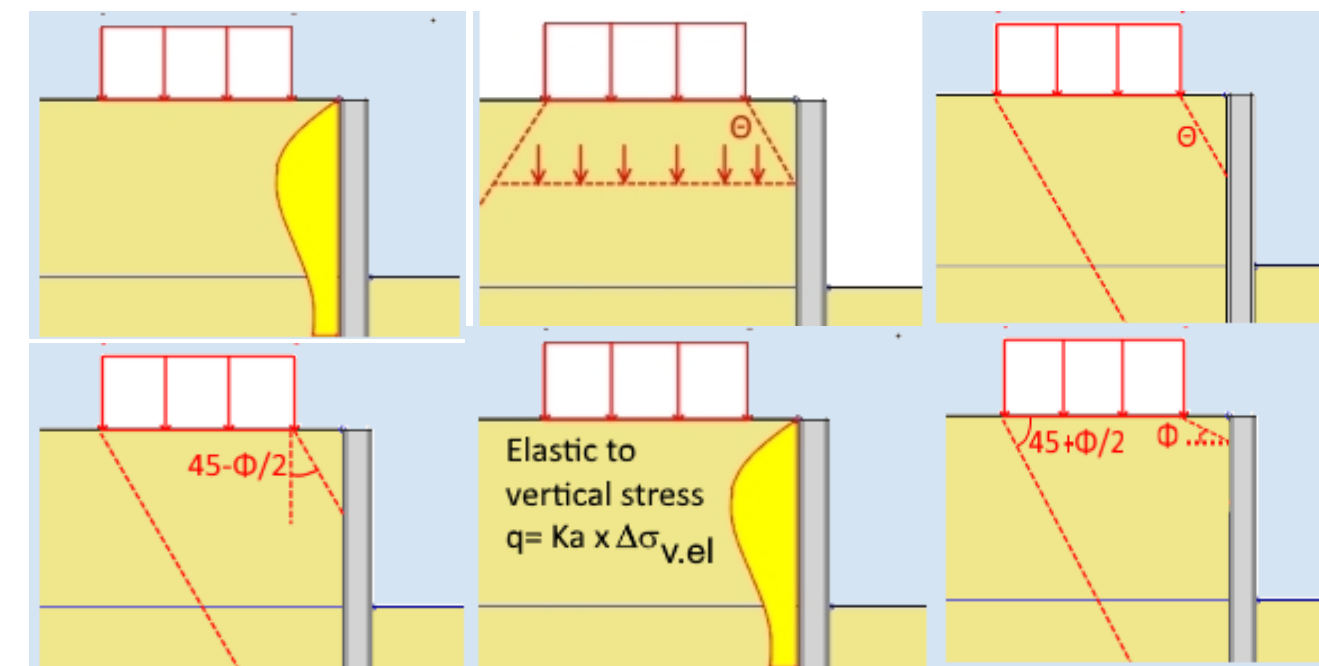
Loads on ground surface:

- ✓ Strip surcharges
- ✓ Linear loads
- ✓ 3D loads (buildings, footings, 3D surface loads)



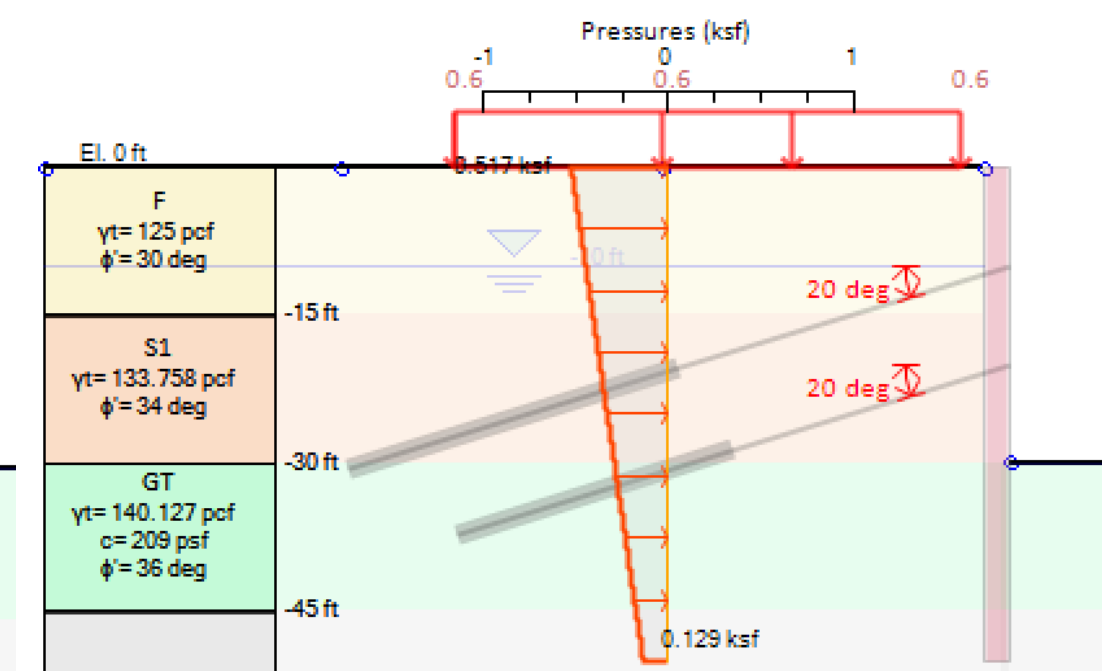
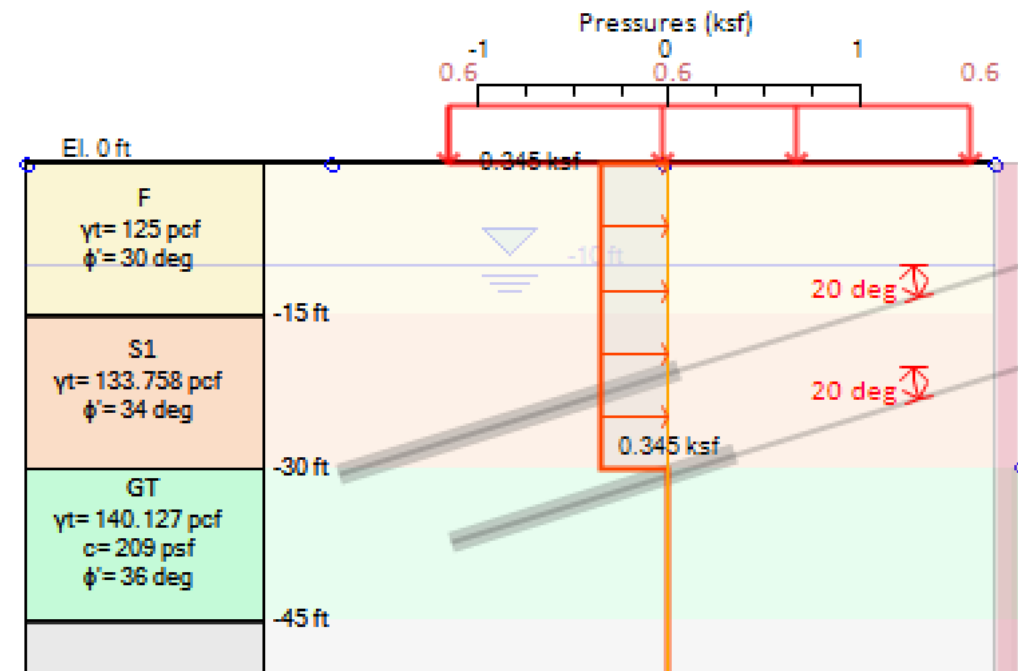
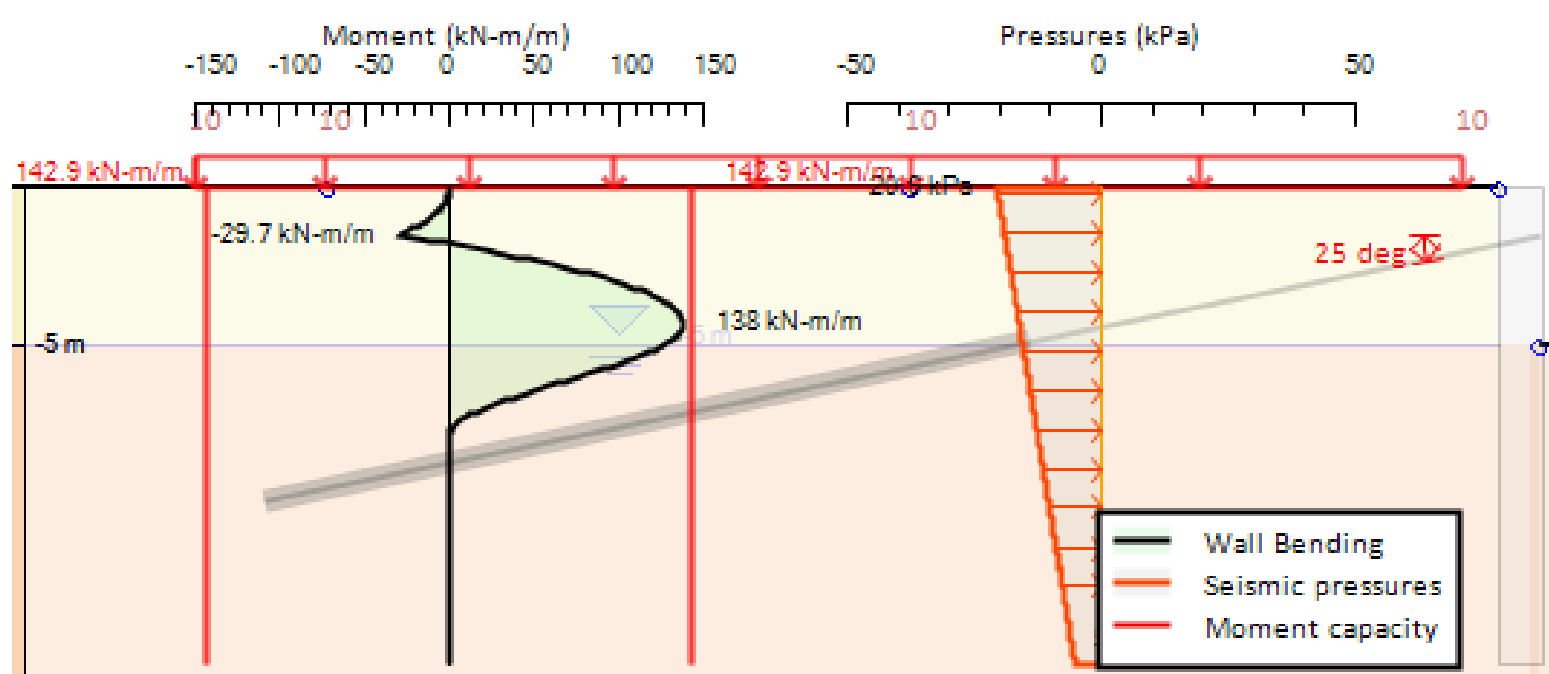
Loads on the wall:

- ✓ Strip surcharges
- ✓ Linear loads
- ✓ External moments
- ✓ Prescribed displacements



Load modeling options:

- ✓ Elasticity equations
- ✓ Two-way distribution angle
- ✓ One-way distribution angle
- ✓ One-way distribution angle from soil friction
- ✓ Elasticity to vertical stress x K_a (or K_o)
- ✓ CIRIA Special Pub 95 - 1993



Procedure in DeepEX

- Define Seismic Accelerations A_x and A_z
- Select Seismic Pressures Calculation Method
- Select a Seismic Design Standard

Seismic Pressure Methods

- ✓ Semirigid
- ✓ Mononobe-Okabe (frictional soils)
- ✓ Wood Automatic
- ✓ Wood Manual

Semirigid Method

- Total Vertical Stress at Bottom of Wall $\times B$
- $B = 0.75$ in DeepEX
- Rectangular Pressure Diagram

Mononobe-Okabe Method (Frictional Soils)

- Extension of the Coulomb Static Theory
- Accelerations added to a Coulomb Wedge
- Seed & Whitman (1970) Seismic Thrust Redistribution
- Inverse Trapezoid Pressure Diagram



Fixed earth method

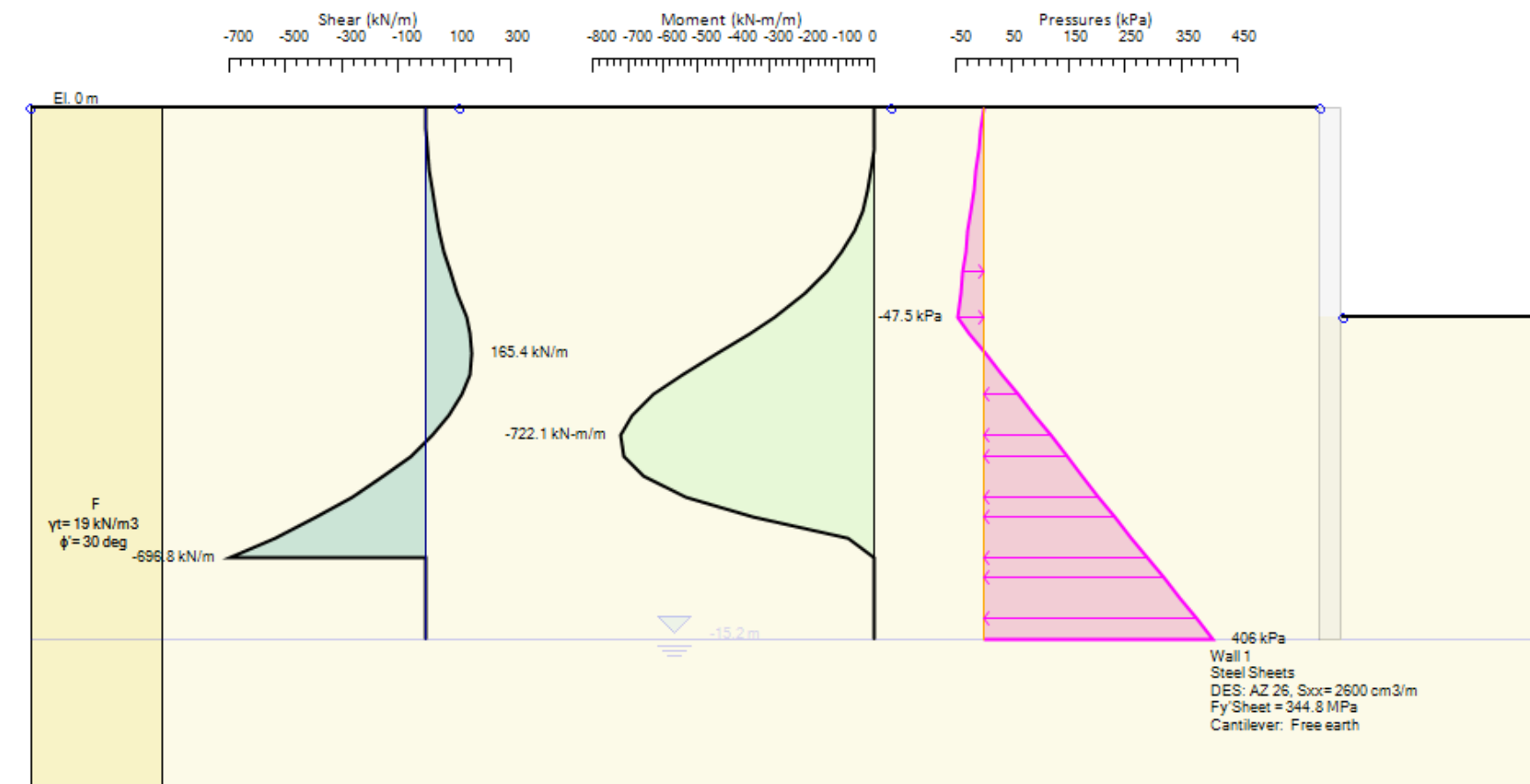
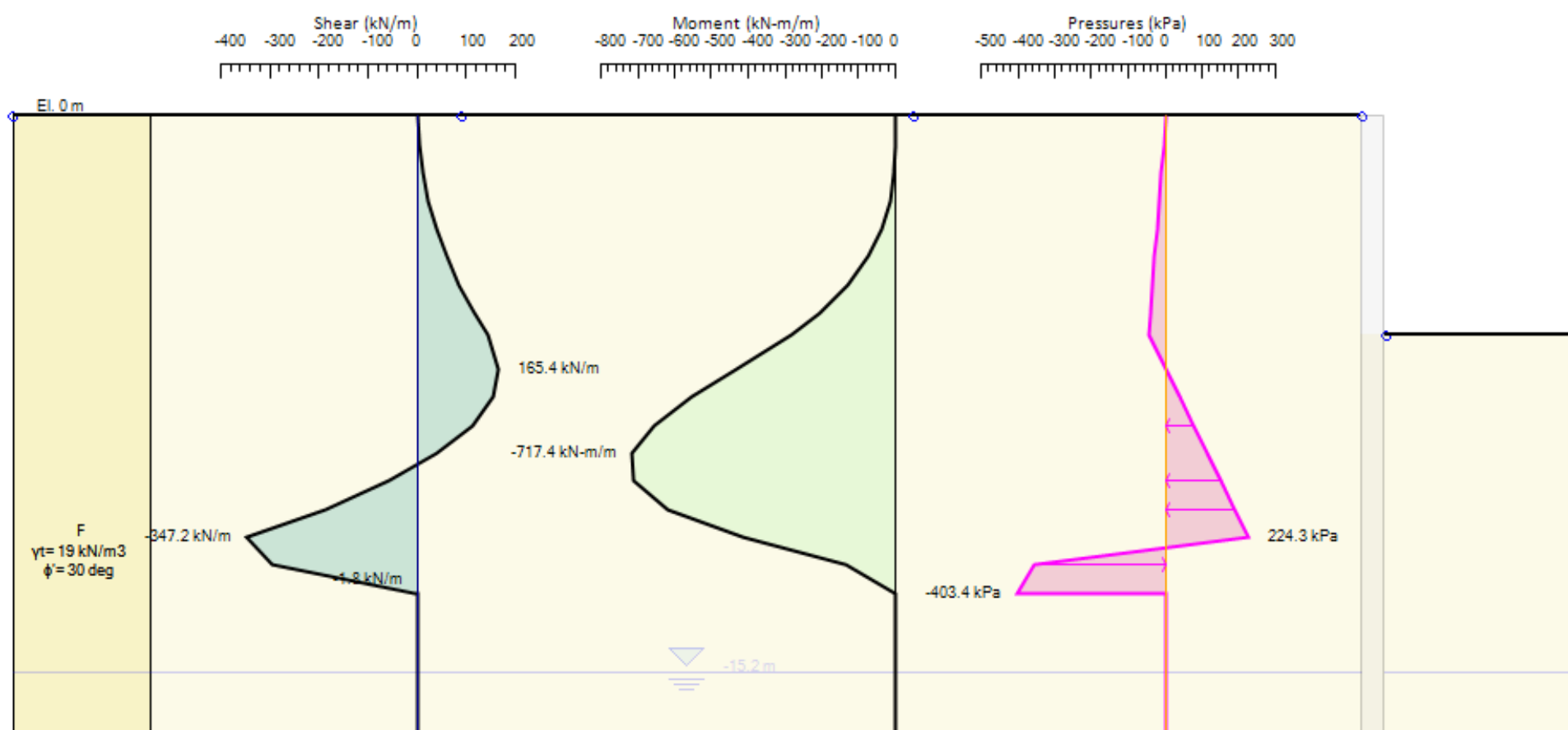
Balances out Moment and Shear

Free earth method

Balances out Moment - Shear not balanced

Increase length by 1.2 to get FS 1.0

Then apply additional safety factors

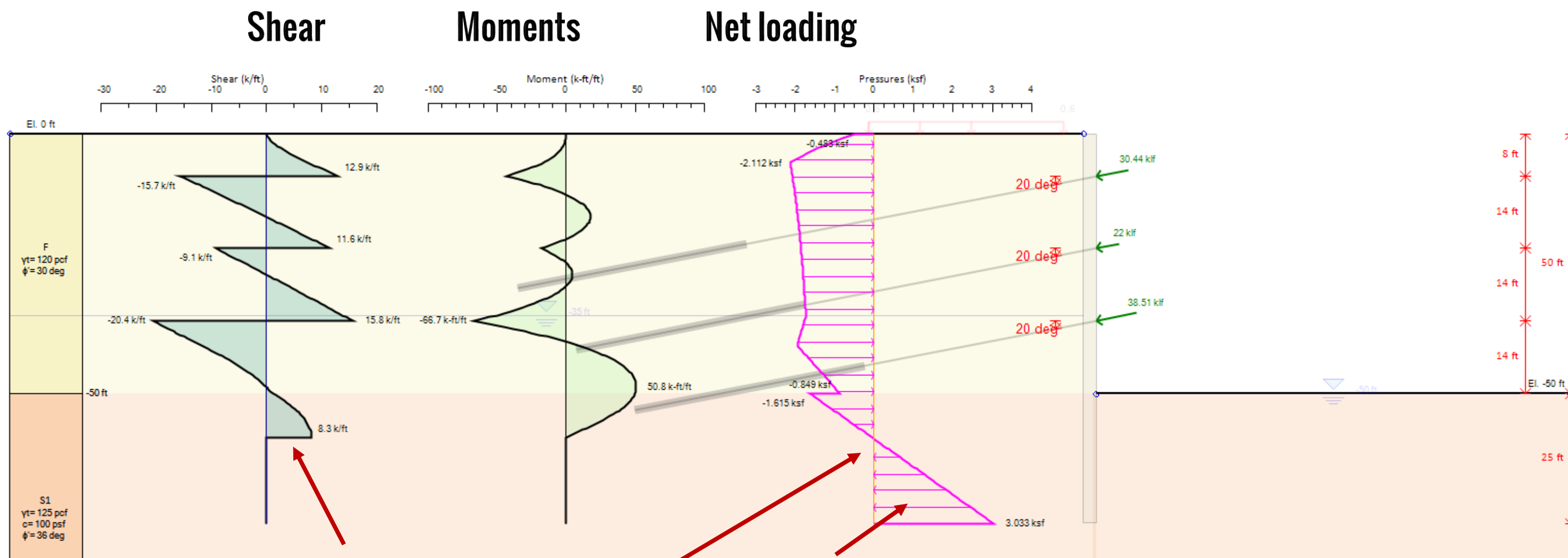




Beam Analysis: Blum's Method



Pinned supports - continuous beam
Point of zero net soil shear below subgrade.
Use point of zero shear as a virtual support.



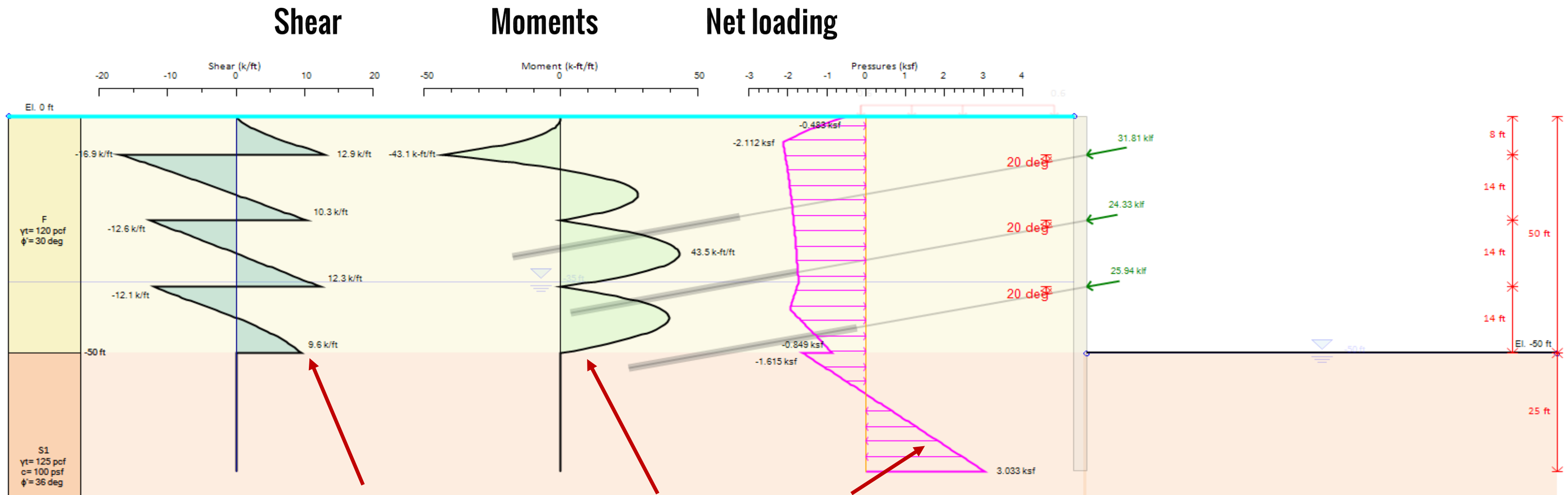
Reaction for embedment F_{xb}

Virtual support Available resistance R_x

$$FS_{\text{passive}} = \frac{R_x}{F_{xb}}$$



Pin support at excavation base, simple spans



Reaction for embedment F_{xb}

Virtual support

Available resistance R_x

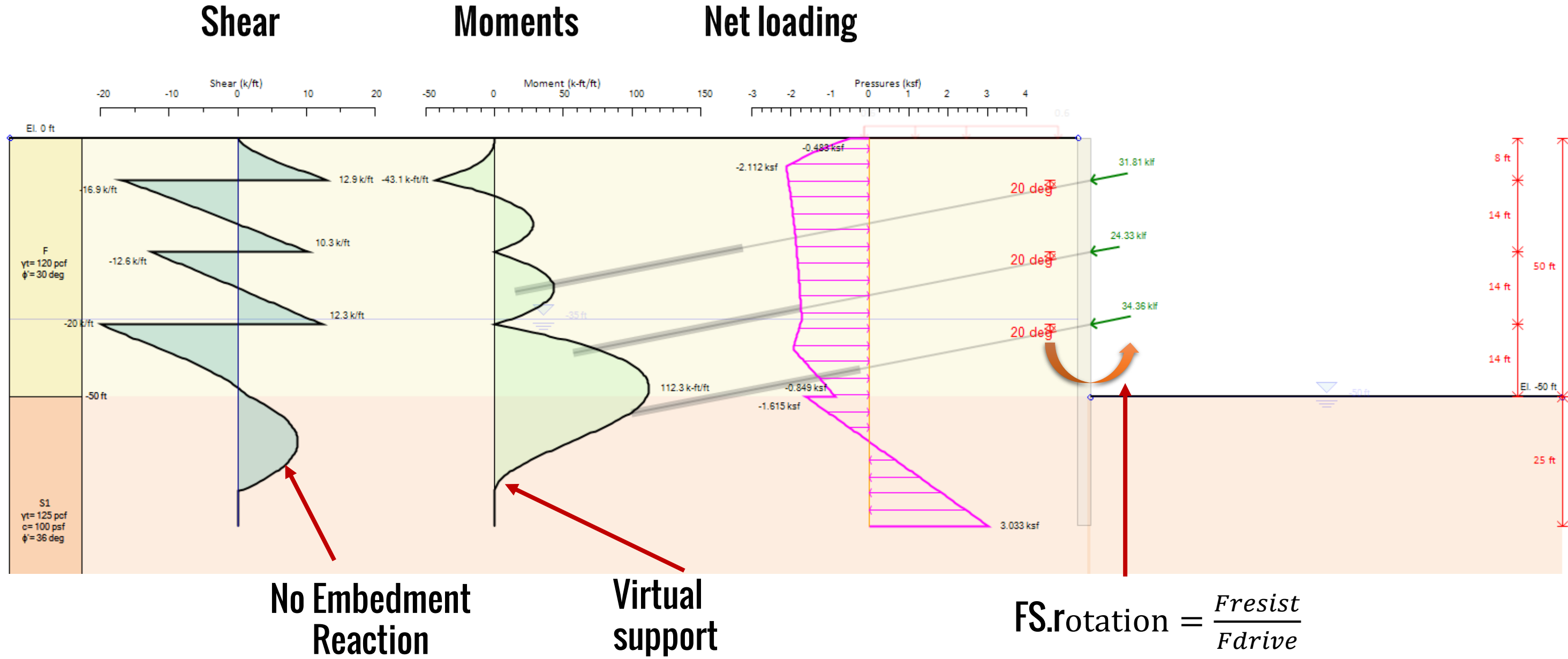
$$FS_{\text{passive}} = \frac{R_x}{F_{xb}}$$



Beam Analysis: CALTRANS Approach

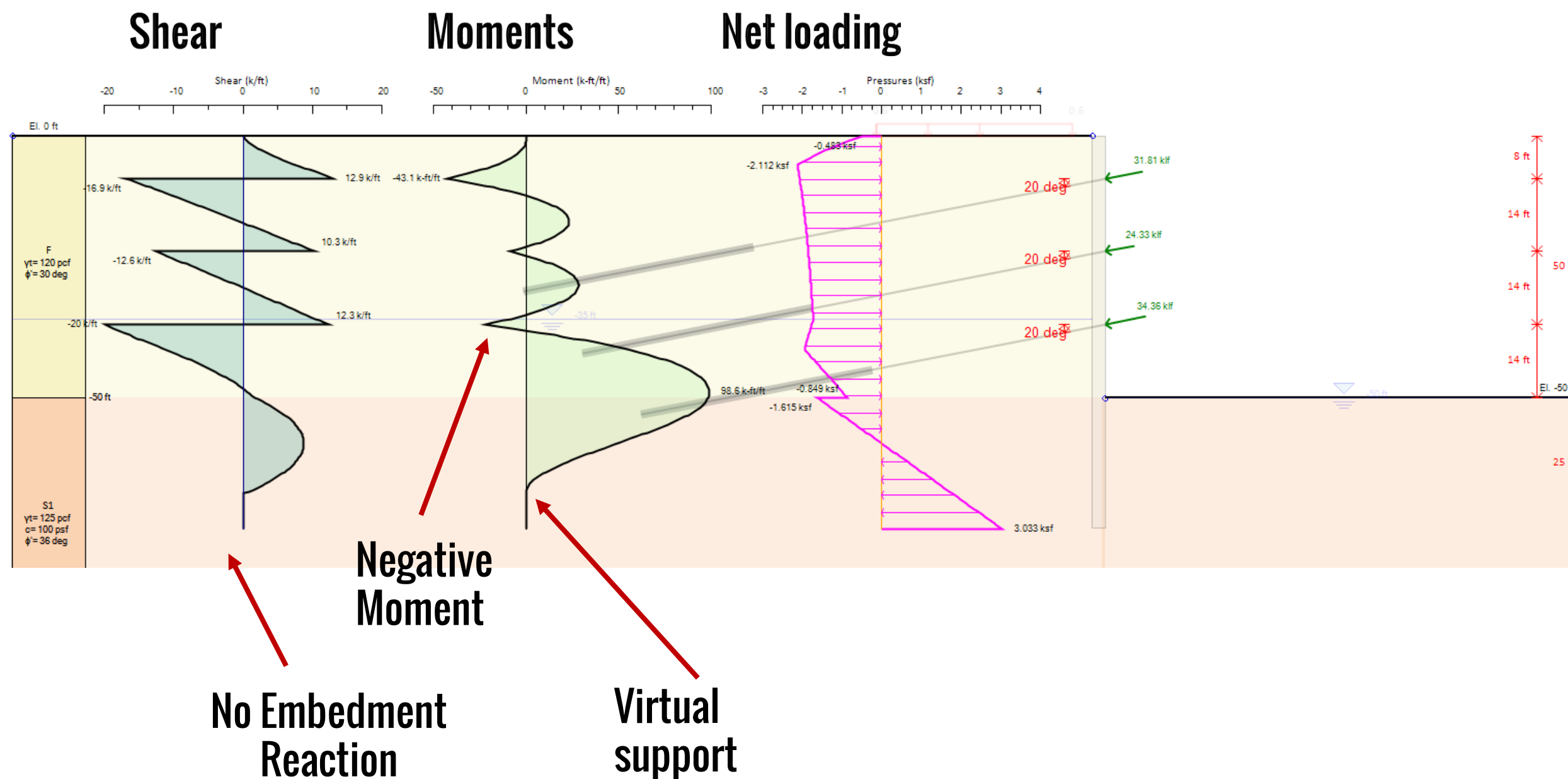


Pinned supports - simple span
Base at point of zero moment below bottom support
Shears and moments balance out

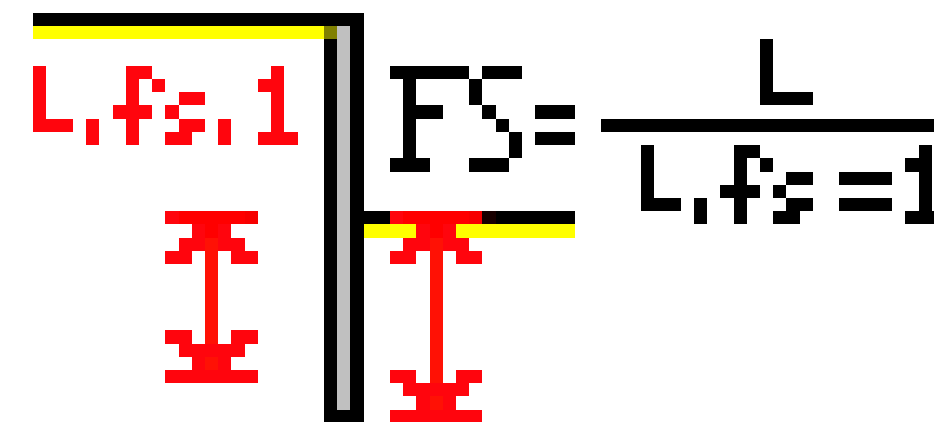
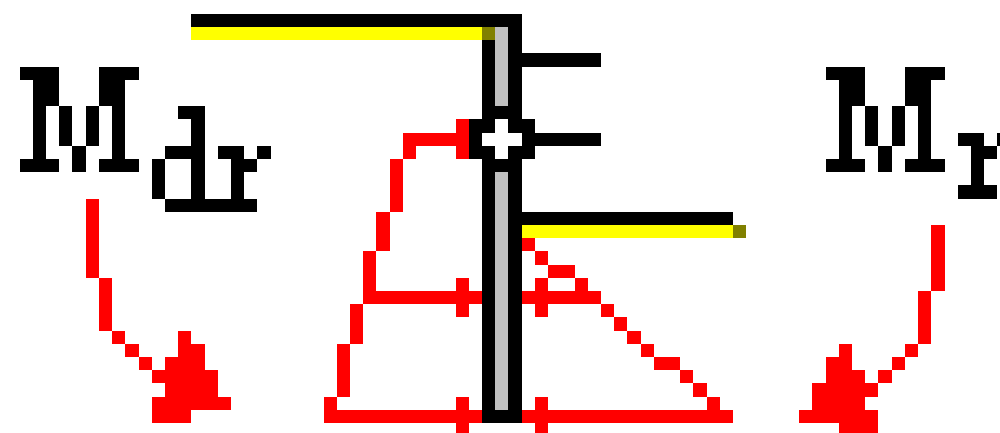
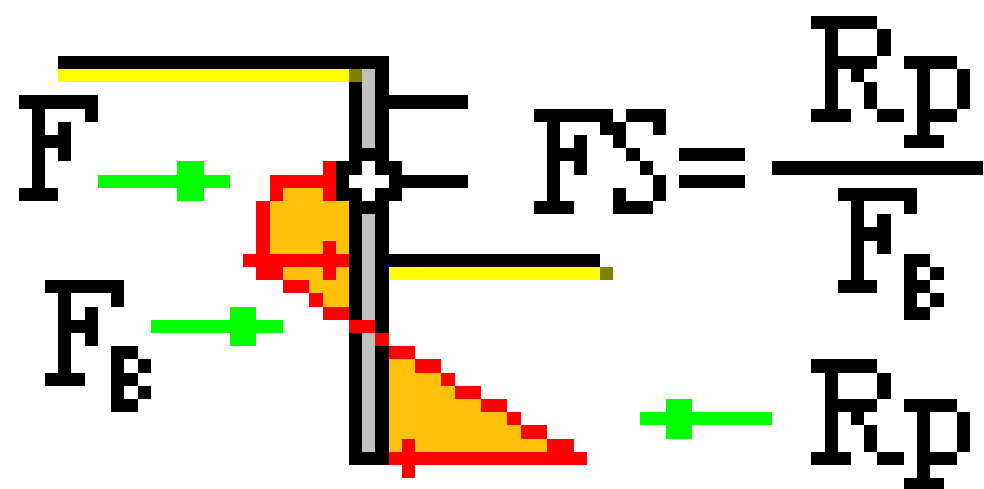




Simple span may be very conservative
Assume negative moments (20% of simple span)



Wall toe Stability checks



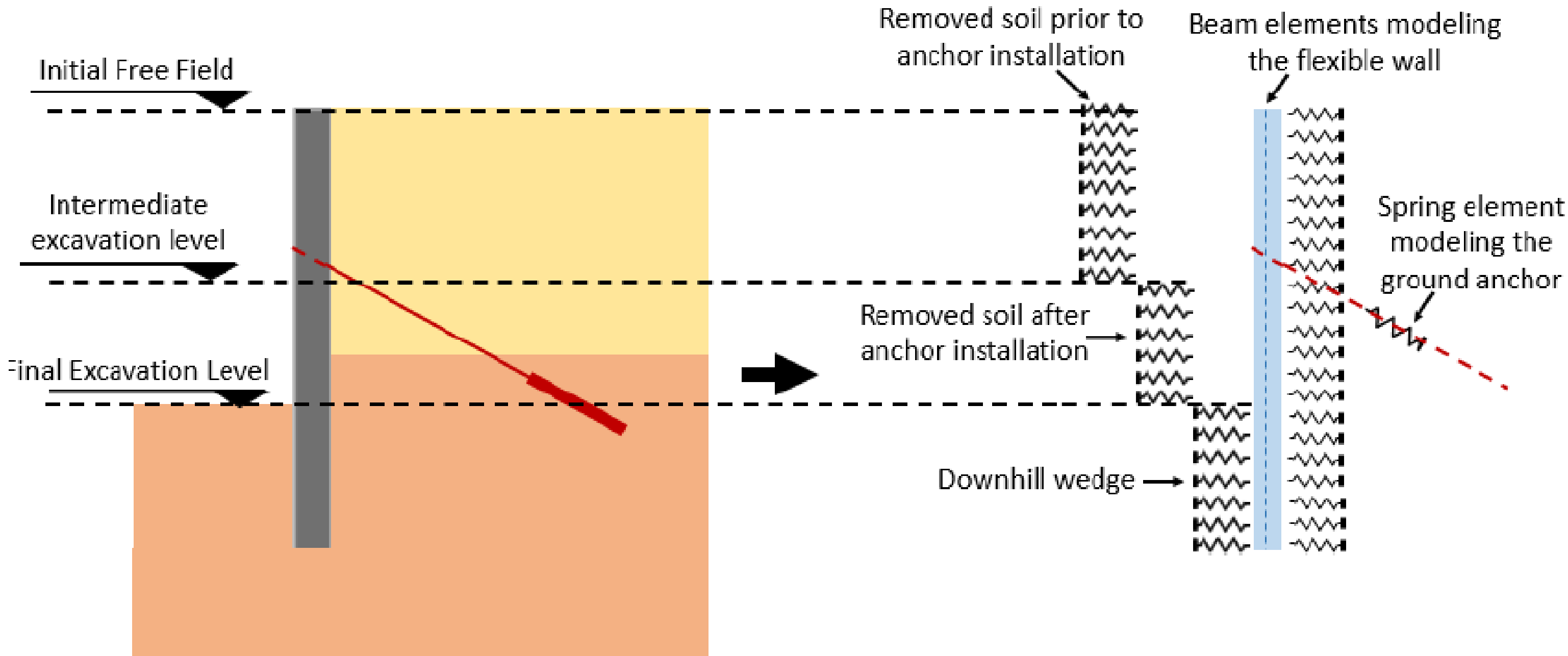
$$FS_{pas} = \frac{\text{Available Resistance beneath virtual fixity point}}{\text{Hor. reaction at virtual point} + \text{driving pressures beneath virtual fixity point}}$$

$$FS_{rotation} = \frac{\text{Resisting moments about a point}}{\text{Driving moments about the same point}} \quad (\text{Eq. 9.2})$$

$$FS_{embed} = \frac{\text{Available wall embedment depth}}{\text{Max. Required embedment depth for } FS = 1 \text{ from Equations 1\& 2 above}} \quad (\text{Eq. 9.3})$$

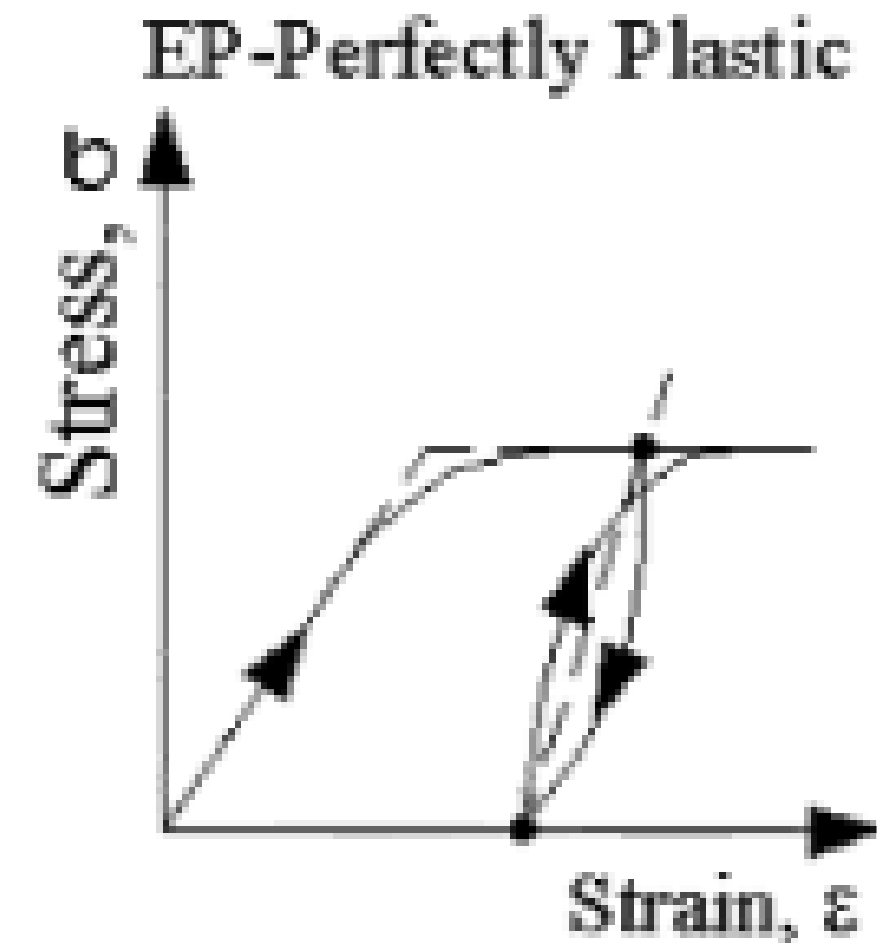


Non-linear Analysis Concept (Soil Springs)



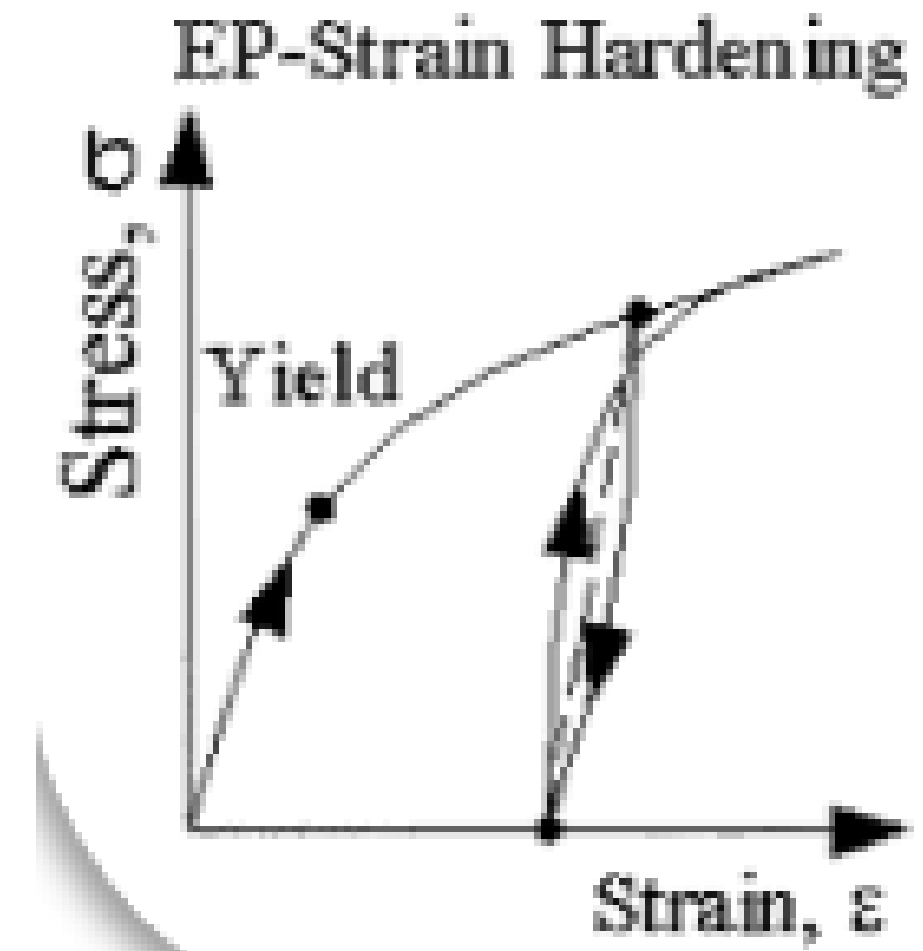
Linear Perfectly plastic Model

- Elastic behavior prior to yielding
- Detailed unloading reloading behavior



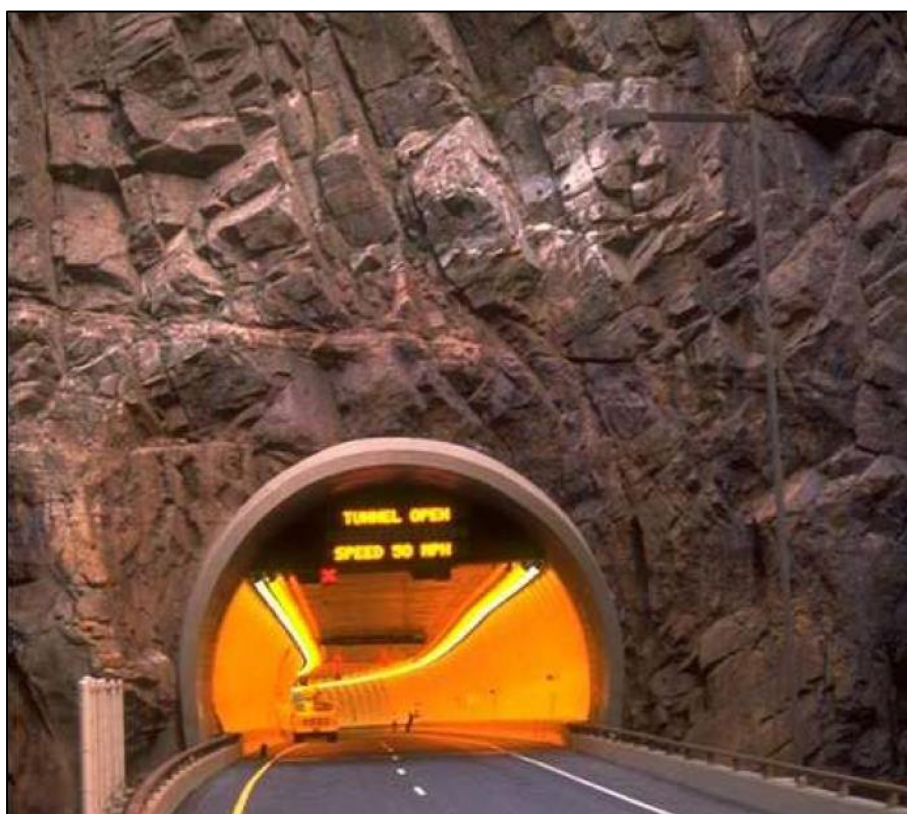
Hyperbolic elastoplastic Model

- Inelastic behavior prior to yielding
- Pressure dependent nonlinear stiffness
- Detailed unloading reloading behavior



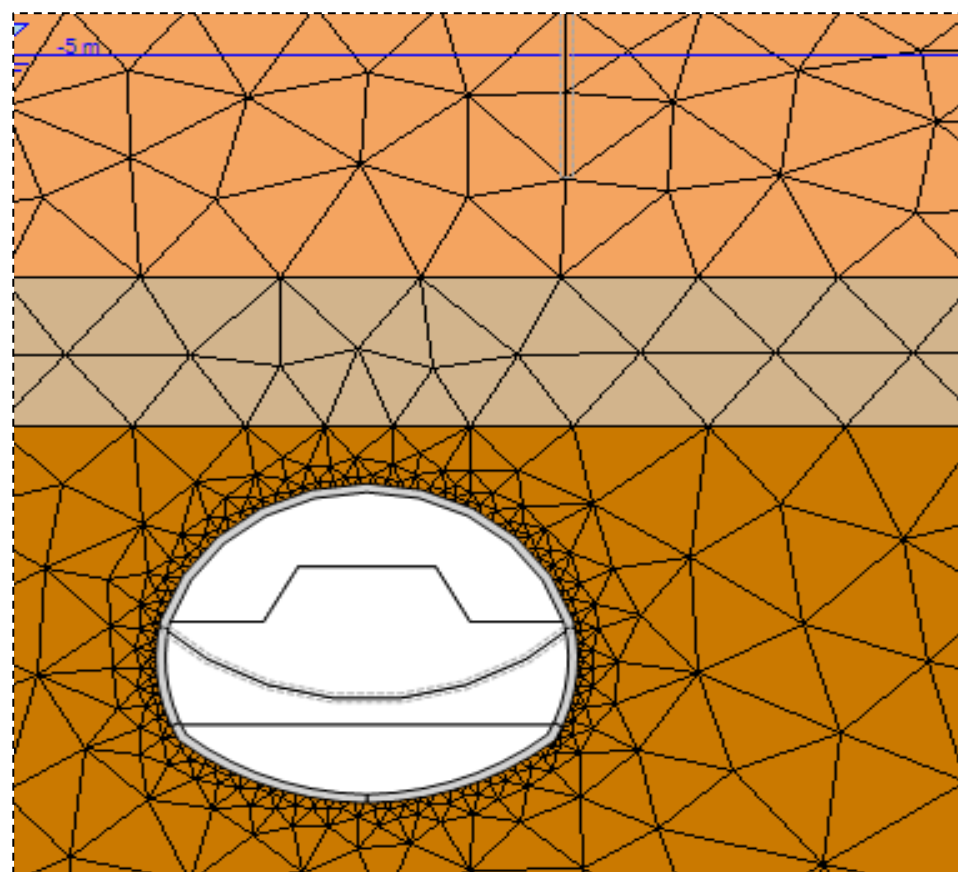
Subgrade Model

- Elastic behavior
- Defined by a K_{subgrade} modulus



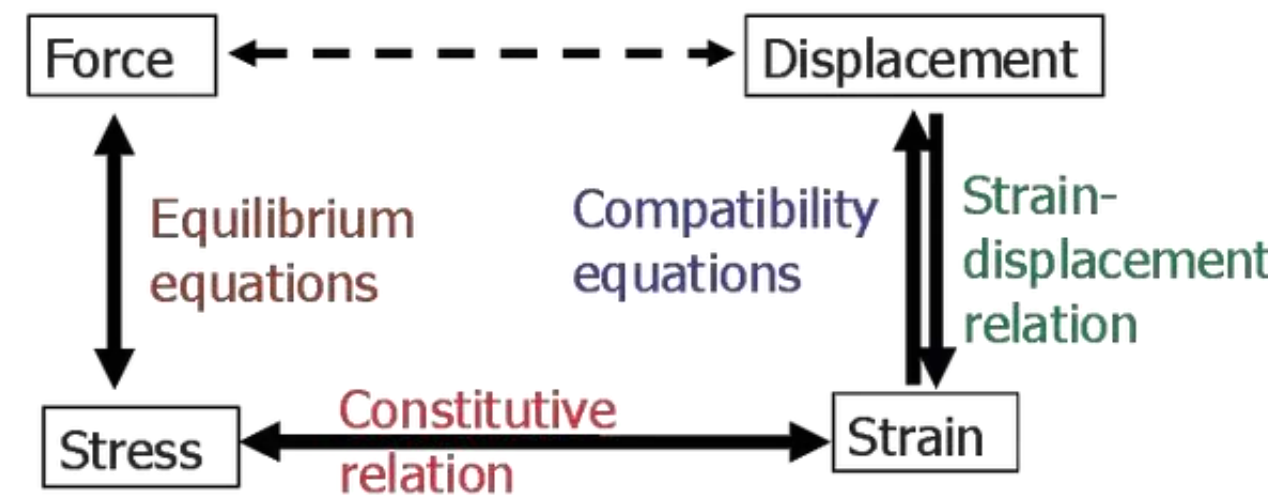
Physical structure

Modelling assumptions



FEM numerical model

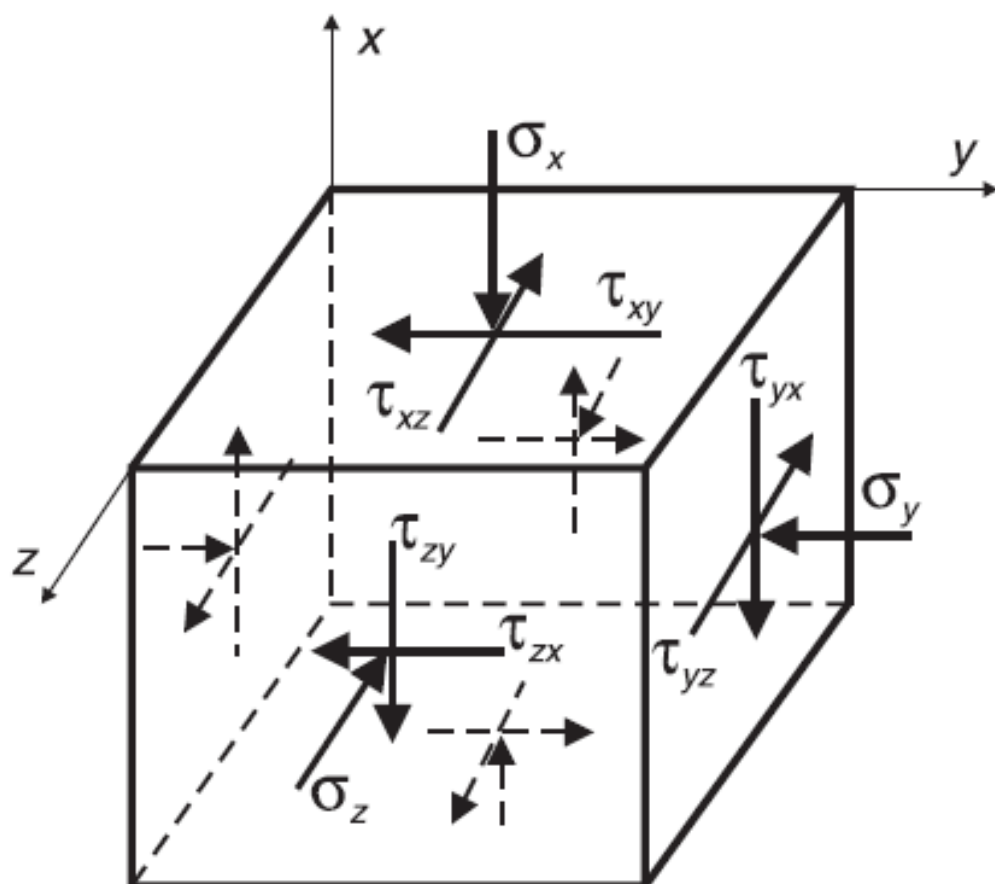
Mathematical formulation



Discretization

(a) Equilibrium equations

Stresses within the soil medium must satisfy equilibrium



Mathematical form: Cauchy momentum equation

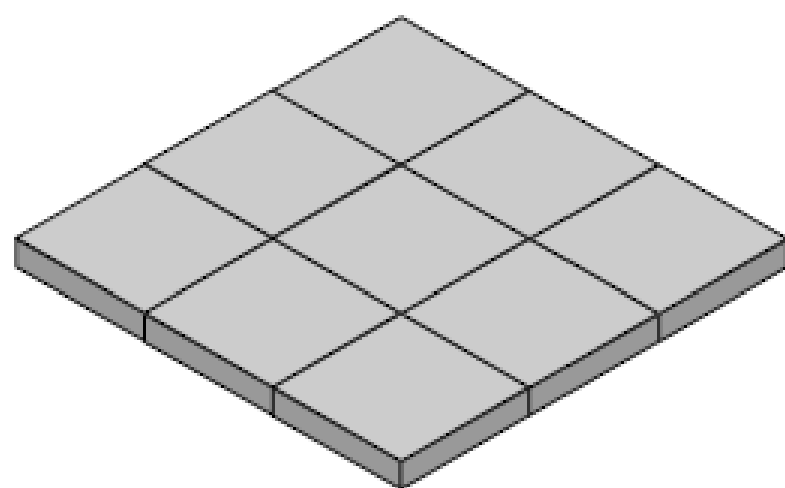
External body forces

$$\text{div}_x \boldsymbol{\sigma} + \mathbf{b} = \rho \ddot{\mathbf{u}}$$

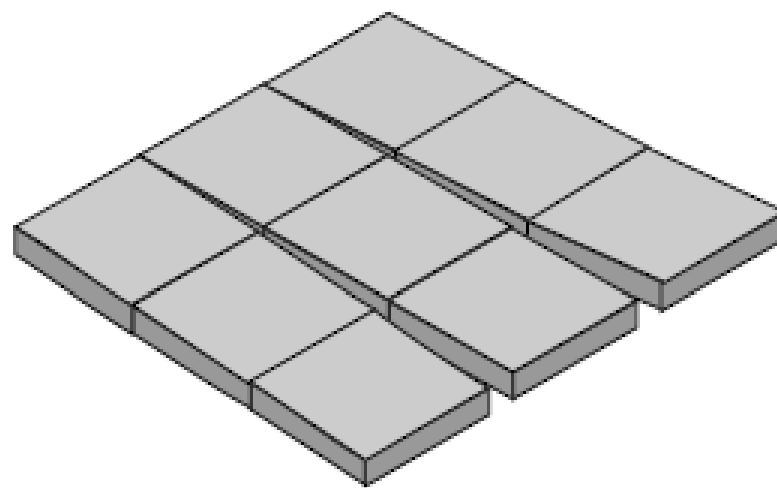
Divergence of stress tensor

Acceleration
(equal to zero for static analysis)

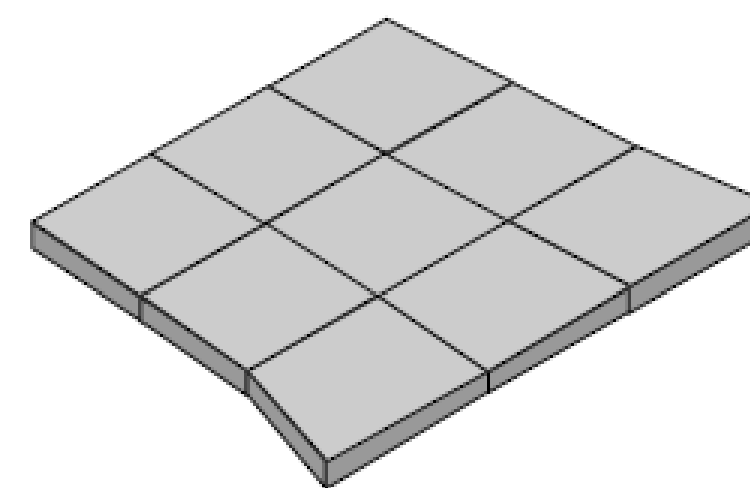
(b) Compatibility equations



(a) Original



(b) Non-compatible

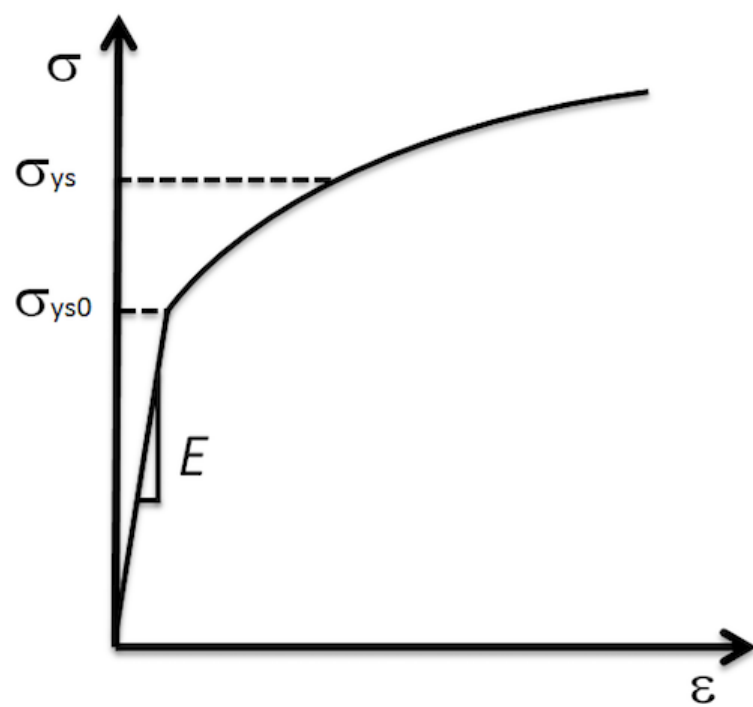


(c) Compatible

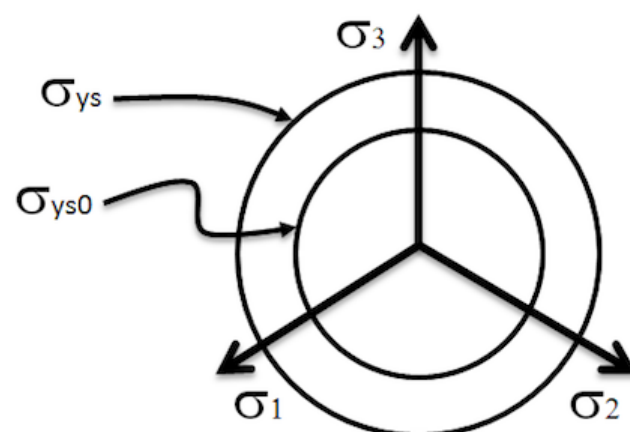
Strain to displacement relationship:

$$\begin{aligned} \epsilon_x &= \frac{\partial u}{\partial x}; & \epsilon_y &= \frac{\partial v}{\partial y}; & \epsilon_z &= \frac{\partial w}{\partial z} \\ \gamma_{xy} &= \frac{\partial v}{\partial x} + \frac{\partial u}{\partial y}; & \gamma_{yz} &= \frac{\partial w}{\partial y} + \frac{\partial v}{\partial z}; & \gamma_{xz} &= \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \end{aligned}$$

(c) Constitutive law



Uniaxial stress-strain curve



Yield surface in principal stress-space

$$\begin{Bmatrix} \Delta\sigma_x \\ \Delta\sigma_y \\ \Delta\sigma_z \\ \Delta\tau_{xy} \\ \Delta\tau_{xz} \\ \Delta\tau_{zy} \end{Bmatrix}$$

Stress
increment

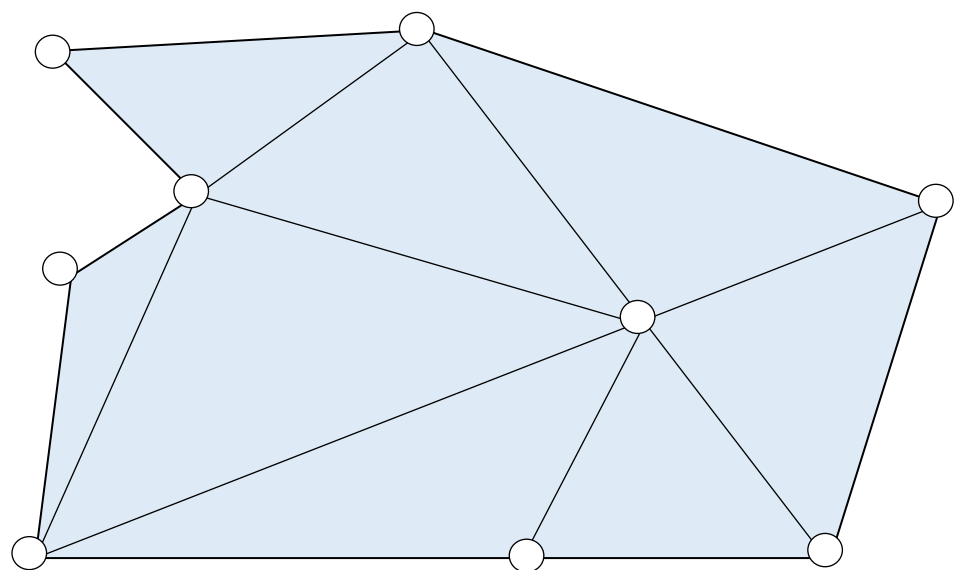
Constitutive law

$$\Delta\sigma = f_m(\Delta\epsilon)$$

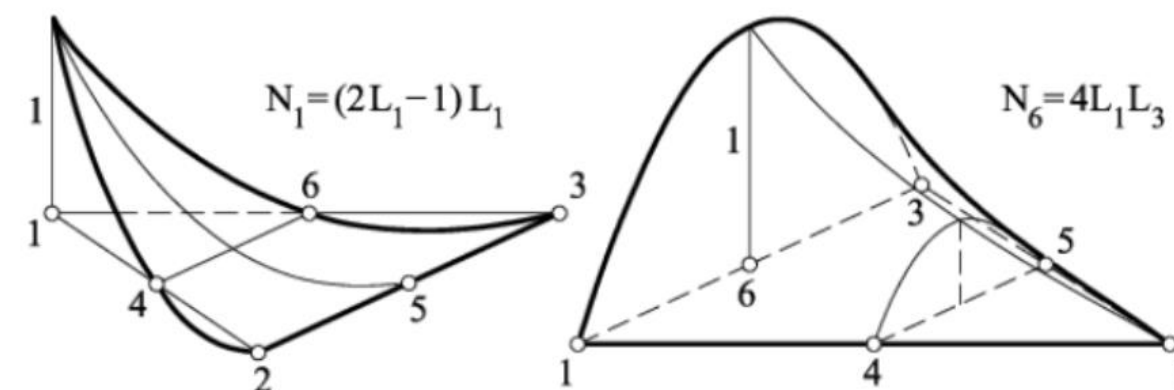
$$\begin{Bmatrix} \Delta\epsilon_x \\ \Delta\epsilon_y \\ \Delta\epsilon_z \\ \Delta\gamma_{xy} \\ \Delta\gamma_{xz} \\ \Delta\gamma_{zy} \end{Bmatrix}$$

Strain
Increment

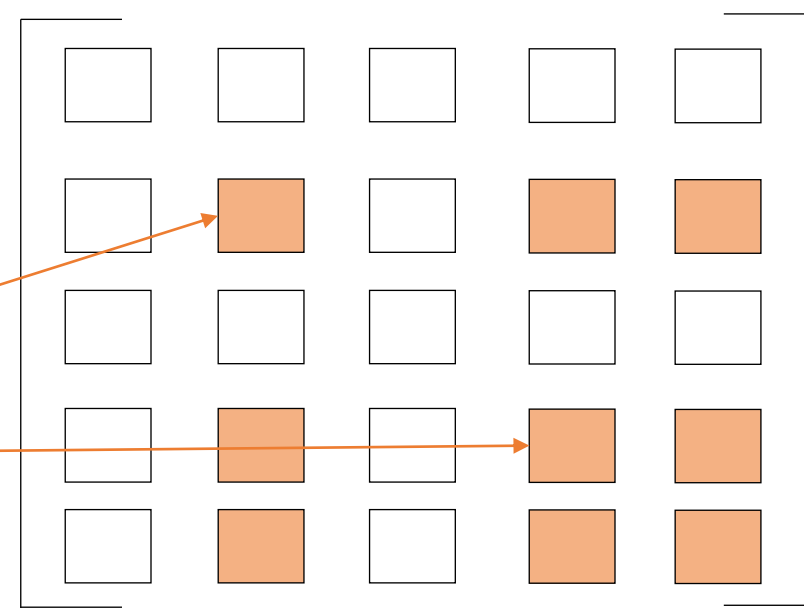
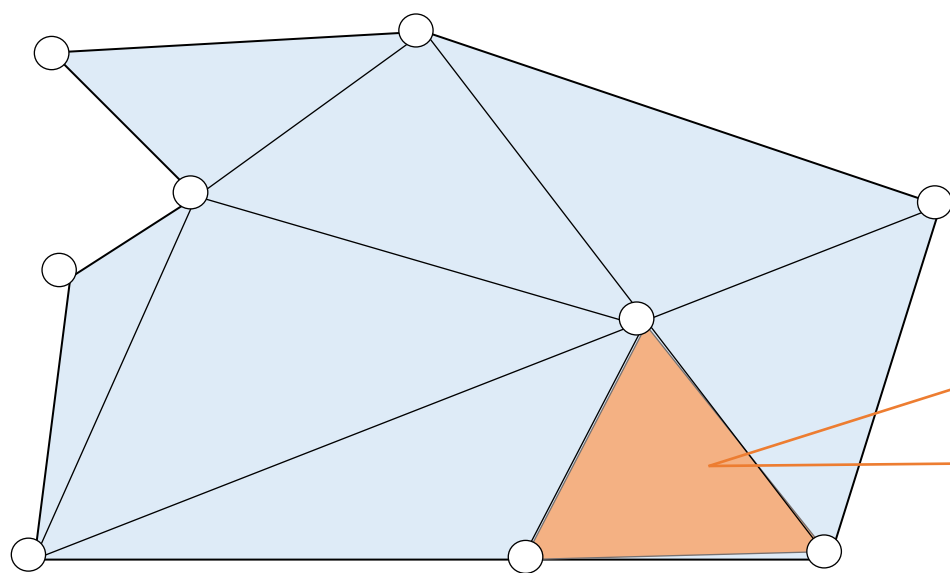
Reformulating the PDE system to an easily solvable system of algebraic equations!!



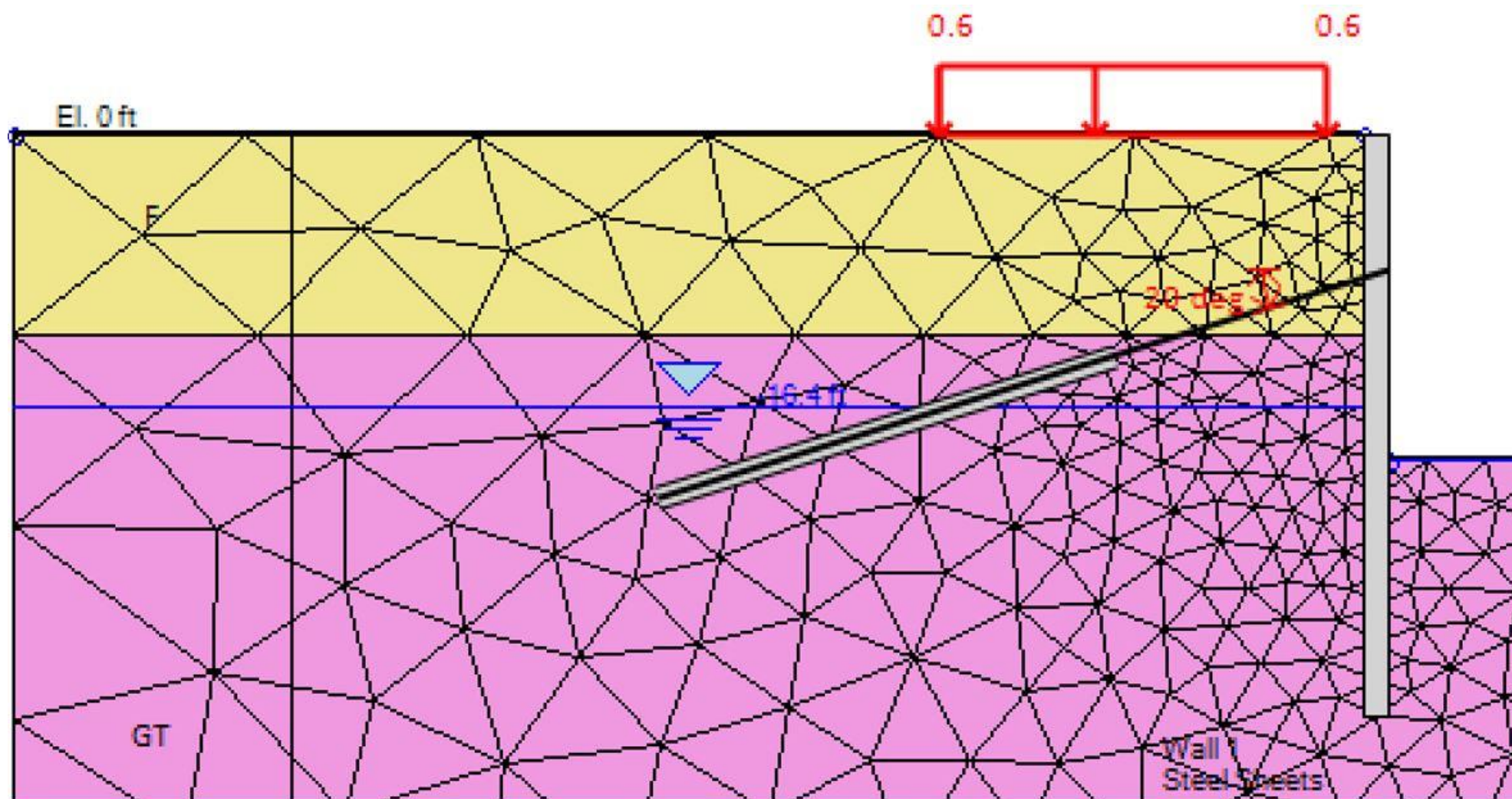
Step 1: Discretize the continuum



Step 2: Select interpolation functions



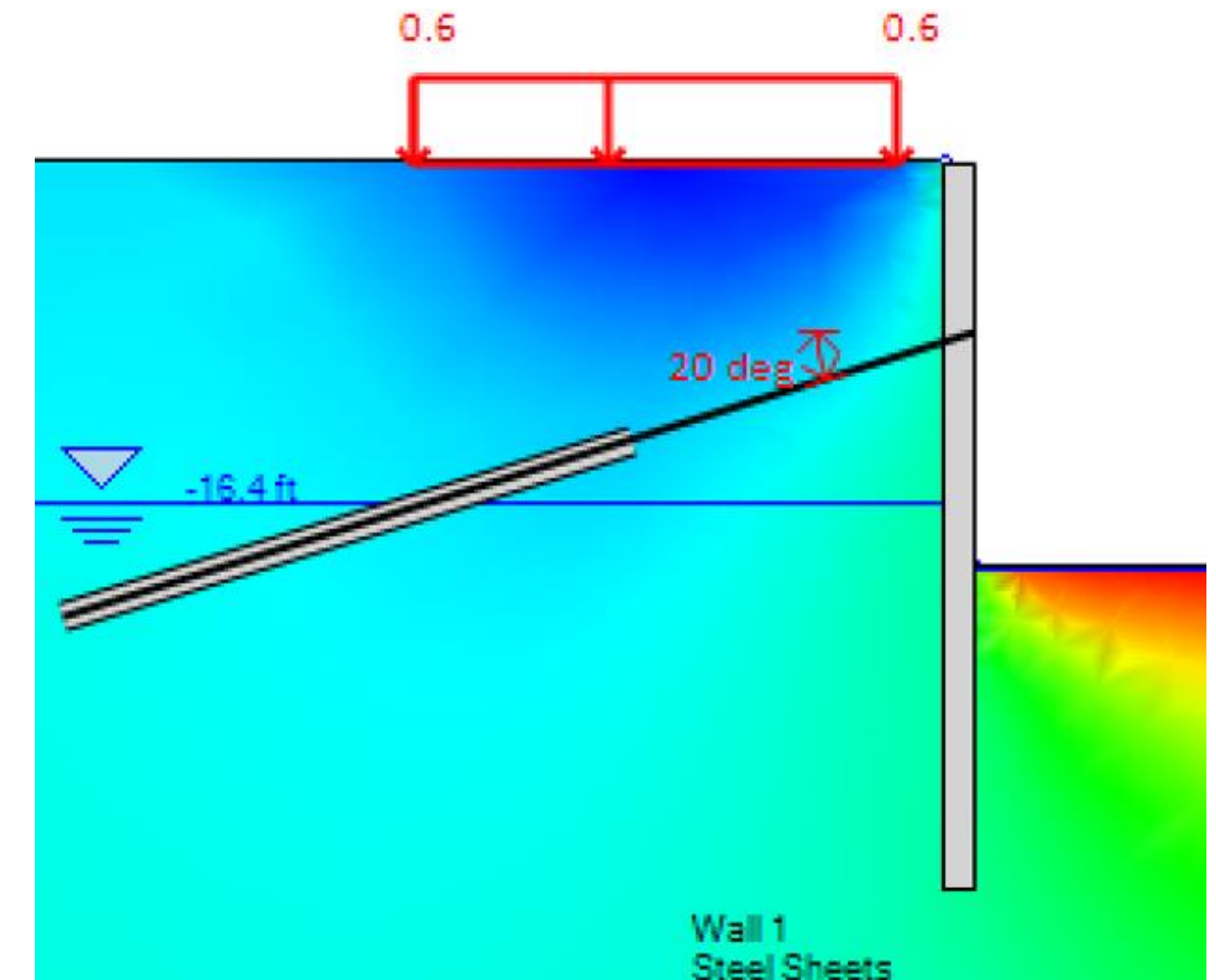
Step 3: Assemble the global equation system



- ✓ Moments and reactions calculated with Finite Elements
- ✓ Consider full soil-structure interaction
- ✓ Calculate surface settlements
- ✓ Design Tiedowns, Foundation Piles and Steel Columns

Soil Models:

- ✓ Elastoplastic Model (Mohr - Coulomb)
- ✓ Exponential (Hyperbolic) Model (approximate solution)
- ✓ Exponential (Hyperbolic) Model (complete solution):
Soil hardening model





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